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Effects of thermo-osmosis on hydraulic behaviour of saturated clays

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Abstract

Despite a body of research carried out on thermally coupled processes in soils, understanding of thermo-osmosis phenomena in clays and its effects on hydro-mechanical behaviour is incomplete. This paper presents an investigation on the effects of thermo-osmosis on hydraulic behaviour of saturated clays. A theoretical formulation for hydraulic behaviour is developed incorporating an explicit description of thermo-osmosis effects on coupled hydro-mechanical behaviour. The extended formulation is implemented within a coupled numerical model for thermal, hydraulic, chemical and mechanical behaviour of soils. The model is tested and applied to simulate a soil heating experiment. It is shown that the inclusion of thermo-osmosis in the coupled thermo-hydraulic simulation of the case study provides a

27 better agreement with the experimental data compared with the case where only thermal
28 expansion of the soil constituents was considered. A series of numerical simulations are also
29 presented studying the pore water pressure development in saturated clay induced by a
30 heating source. It is shown that pore water pressure evolution can be considerably affected by
31 thermo-osmosis. Under the conditions of the problem considered, it was found that thermo-
32 osmosis changed the pore water pressure regime in the vicinity of the heater in the case where
33 value of thermo-osmotic conductivity was larger than $10^{-12} \text{ m}^2 \cdot \text{K}^{-1} \cdot \text{s}^{-1}$. New insights into the
34 hydraulic response of the ground and the pore pressure evolution due to thermo-osmosis are
35 provided in this paper.

36

37 **Keywords**

38 Thermo-osmosis; saturated clay; hydraulic behaviour; coupled modelling; hydro-mechanical

39

40

41

42 **Introduction**

43 Temperature variations can induce various processes and changes in soil-water system and
44 affect the engineering behaviour of soil. In many engineering applications, considerable
45 changes in ground temperature can be expected. Examples are i) the geological disposal of
46 high level radioactive waste where elevated temperature is generated by the waste and ii)
47 ground source heat or energy foundations where thermal gradients in the soil are induced by
48 exchanging heat with the ground. Understanding the processes and parameters involved in
49 the behaviour of soils under non-isothermal conditions is therefore of importance for
50 performance analysis and sustainable design of various geo-structures.

51 It has been previously shown that heating of soils can cause pore water pressure development
52 and contraction/dilation in the soil depending on the stress history (Campanella and Mitchell
53 1968; Towhata et al. 1993; Sultan et al. 2002; Laloui and François 2009). Observations from
54 a field experiment of heat storage in clay have indicated that by applying a temperature
55 gradient, an excess pore water pressure has developed during the heating period which has
56 resulted in a considerable settlement after pore water pressure dissipation (Moritz and
57 Gabriëlsson 2001). The difference between the thermal expansion properties of water and
58 solid particles of soil has been described as being the most influential mechanism for pore
59 water pressure development in saturated soils (Mitchell 1993).

60 Osmosis phenomena are among key processes controlling the water flow, chemical transport
61 and deformation behaviour of clays. Under non-isothermal conditions, thermo-osmosis is a
62 coupled mechanism that describes fluid flow in saturated clays under a temperature gradient.
63 From a thermodynamics point of view, thermo-osmosis is controlled by the enthalpy
64 difference between the free water/fluid in the clay pores and the pore water affected by the

65 clay interactions (Gonçalvès and Trémosa 2010). In particular, the presence of an electric
66 field normal to the clay mineral surface modifies the structure and properties of the fluid at
67 the solid surface which causes an alteration of the specific enthalpy of the solution in the pore
68 space. Due to such alteration, a positive change in enthalpy can cause a liquid flow from the
69 warm to the cold side of a sample (Gonçalvès and Trémosa 2010). Despite a significant body
70 of research studying the hydraulic behaviour of saturated and unsaturated clays, limited
71 studies have discussed the effects of thermo-osmosis on the overall thermo-hydro-mechanical
72 behaviour of saturated soils.

73 The soil property associated with thermo-osmosis is expressed by the thermo-osmotic
74 conductivity which has been reported to be in the range of 10^{-14} - 10^{-10} $\text{m}^2\cdot\text{K}^{-1}\cdot\text{s}^{-1}$ (Dirksen
75 1969; Soler 2001; Zheng and Samper 2008; Ghassemi et al. 2009). Due to the wide range of
76 values reported in the literature, the impact of thermo-osmotic conductivity value on pore
77 water pressure evolution and fluid flow near a heat source is still unclear. While it can be
78 expected that the effect of thermo-osmosis on pore water pressure evolution becomes more
79 significant with an increase in the magnitude and duration of a heat emission due to the
80 established thermal gradient in the vicinity of the heat source, further understanding on the
81 importance of thermo-osmosis on such behaviour is required.

82 Experimental investigations have demonstrated the effects of thermo-osmosis on water flow
83 under temperature gradient in saturated clays (e.g. Dirksen 1969; Trémosa et al. 2010).
84 Dirksen (1969) has reported the flow of water in compacted saturated clays induced by
85 applying a temperature gradient. Trémosa et al. (2010) have reported a series of in-situ
86 experiments where the importance of thermo-osmosis on water flow in clay-rock has been
87 examined. The results have suggested that thermo-osmosis can affect the water flow and
88 contribute to the excess pore pressure observations in impervious rocks. Gonçalvès et al.

89 (2012) have reported that in low porosity saturated clays, the water flow induced by a
90 temperature gradient occurs from the warm boundary to the cold boundary whilst in high-
91 porosity soils with a weak thermo-osmotic property, the water flow can occur in the opposite
92 direction of the temperature gradient.

93 Recent studies have indicated that the electro-chemical interactions in the water-clay
94 interface correlate with thermo-osmotic properties (Gonçalvès and Trémosa 2010; Gonçalvès
95 et al. 2012). An expression for the thermo-osmotic conductivity considering the enthalpy
96 change due to hydrogen bonding to the clay surfaces has been suggested by Gonçalvès et al.
97 (2012). Correlations have been provided between thermo-osmotic permeability and intrinsic
98 permeability for different clays which indicate that the ratio of thermo-osmotic conductivity
99 to the intrinsic permeability is strongly correlated with the electrochemical properties of the
100 soil (Gonçalvès et al. 2012). For most of the soils, the ratio falls within a range of 10^{-10} to 10^{-3}
101 Pa.K^{-1} , while for some shales it can be up to 10^8 Pa.K^{-1} (Gonçalvès et al. 2012).

102 A limited number of modelling studies exist describing the effects of thermo-osmosis on
103 coupled thermo-hydraulic and thermo-hydro-mechanical behaviour of saturated clays. A
104 coupled thermo-poroelastic numerical investigation on semi-impermeable clay barriers has
105 been presented by Zhou et al. (1998) which considers compressibility, thermal expansion of
106 constituents and thermo-osmosis. Sánchez et al. (2010) have studied the thermo-hydro-
107 mechanical behaviour based on the results of a mock-up heating test with a special emphasis
108 on the effect of the thermo-osmotic flow in the hydration of the clay barrier. The results of
109 this study have indicated that the inclusion of the thermo-osmotic flow in the analysis of
110 moisture migration has improved the accuracy of the model prediction. Zheng et al. (2011)
111 have presented a numerical investigation of the coupled thermal, hydraulic and chemical
112 behaviour of compacted clay including the effects of thermo-osmosis. The results of

113 sensitivity analyses presented by Zheng et al. (2011) have indicated that the water contents
114 and dissolved concentrations are strongly sensitive to the intrinsic permeability and the
115 thermo-osmotic permeability. Furthermore, authors have concluded that chemical osmosis is
116 of less importance comparing to Darcian flux and thermo-osmosis. An analysis of the pore
117 pressure distribution in shale formations under thermal, hydraulic, chemical and electrical
118 interactions has been conducted by Roshan and Aghighi (2012). Authors have shown that
119 thermo-osmosis plays a considerable effect on the pore pressure evolution when considering
120 the temperature gradient between the formation and the drilling fluid. Chen et al. (2013) have
121 studied the effect of positive and negative thermo-osmotic coefficients on pore water pressure
122 evolution in low porous material. A conclusion has been made that thermo-osmosis can have
123 a strong influence on water transport and mechanical deformation. Further research needs
124 have been suggested as a result of this work to obtain a better understanding of the direction
125 of the water flow in the domain.

126 More recently, Yang et al. (2014) have presented an analytical model for the coupled effect
127 of thermo-osmosis in saturated porous medium and studied the pore pressure and stress
128 variations of clay under thermal effects. Similarly, Roshan et al. (2015) have developed an
129 analytical solution for a thermo-osmotic test that includes the effects of the solid-thermal
130 expansion and thermo-osmosis. By conducting a series of thermo-osmotic experiments and
131 further analysis using the analytical model, authors have concluded that for the correct
132 interpretation of experimental data, it is important that both solid-fluid thermal expansion and
133 thermo-osmosis are considered in the analysis.

134 This paper presents the development of a formulation and a numerical model for coupled
135 thermal, hydraulic and mechanical behaviour of saturated clay with emphasis on studying the
136 effects of thermo-osmosis on hydraulic behaviour. A formulation of water flow is presented

137 which is implemented within an existing numerical model of coupled thermal, hydraulic,
138 chemical and mechanical behaviour of unsaturated soils (Thomas and He 1997; Thomas et al.
139 2003; Sedighi et al. 2016). The results of a series of numerical investigations are presented to
140 test the implementation of the formulation in the model. The effects of thermo-osmosis are
141 considered by comparison with analytical and experimental benchmarks. In addition, a series
142 of numerical simulations are presented which aim to investigate the effects of thermo-
143 osmosis on pore water pressure development around a heat source.

144

145 **Theoretical formulation and numerical model**

146 The formulation presented below is based on the general formulation of coupled thermal,
147 hydraulic and mechanical behaviour (THM) for unsaturated soils presented by Thomas and
148 He (1997). A simplified form of the coupled THM formulation for saturated clays is extended
149 here which considers the effects of thermo-osmosis in hydraulic behaviour. It is noted that the
150 focus of the paper is on the effects of thermo-osmosis on hydraulic behaviour; hence a
151 simplified mechanical formulation is adopted and included in the governing equations for
152 water flow. The system is fully saturated; therefore the vapour flow is not considered.

153 **Hydraulic behaviour**

154 The governing equation of water flow is considered based on the principle of mass
155 conservation and following the formulation provided by Thomas and He (1997). This can be
156 expressed as (Thomas and He 1997):

$$\frac{\partial}{\partial t}(\rho_l \theta_l \delta V) = -\delta V \nabla \cdot (\rho_l v_l) \quad (1)$$

157 where t is time, ρ_l is the density of liquid water, θ_l is the volumetric water content, ∇ is the
 158 gradient operator and δV is the incremental volume of the soil. v_l is the velocity of liquid that
 159 is calculated based on Darcy's law for water flow in saturated soils.

160 Expanding equation (1) for fully saturated clay with respect to its partial derivatives and
 161 using an incremental volume as a summation of the void volume and solid volume yields:

$$e\delta V_s \frac{\partial \rho_l}{\partial t} + \rho_l \delta V_s \frac{\partial e}{\partial t} + \rho_l e \frac{\partial (\delta V_s)}{\partial t} = -\delta V_s (1 + e) \nabla \cdot (\rho_l v_l) \quad (2)$$

162 where δV_s is the increment volume of the solids and e stands for the void ratio of soil.

163 Dividing equation (2) by δV_s and $(1 + e)$, yields:

$$n \frac{\partial \rho_l}{\partial t} + \rho_l \frac{\partial e}{\partial t (1 + e)} + \rho_l n \frac{1}{\delta V_s} \frac{\partial (\delta V_s)}{\partial t} = -\nabla \cdot (\rho_l v_l) \quad (3)$$

164 For a deformable soil the term $\partial e / \partial t (1 + e)$ can be expressed as $\partial \varepsilon_v / \partial t$, where ε_v is the
 165 volumetric strain that is, by definition, the rate of change of the void ratio with respect to the
 166 initial volume. The total volumetric elastic strain should include the contributions of thermal
 167 expansion and effective stress which can be presented as (Thomas and He 1997; Hueckel et
 168 al. 2011); therefore:

$$\frac{\partial \varepsilon_v}{\partial t} = \alpha \frac{\partial T}{\partial t} - m_v \frac{\partial \sigma'}{\partial t} \quad (4)$$

169 where α is the thermal expansion coefficient of the soil structure, m_v is the coefficient of soil
 170 compressibility, T is the temperature and σ' is the effective stress which is based on
 171 Terzaghi's effective stress principle equal to $\sigma' = \sigma - u_l$ where σ is the total stress and u_l is
 172 the pore water pressure. Thermal expansion coefficient of the soil structure is a negative
 173 value if an increase in temperature causes a decrease in volume (Mitchell 1993; Thomas et al.
 174 1996).

175 Assuming a constant mass of the solid phase, the temporal variation of the solid volume is
 176 equal to the temporal variation of the solid density, given as:

$$\frac{1}{\delta V_s} \frac{\partial(\delta V_s)}{\partial t} = -\frac{1}{\rho_s} \frac{\partial(\rho_s)}{\partial t} \quad (5)$$

177 where ρ_s is density of the solid phase.

178 The density of liquid phase can be presented as a function of water pressure and temperature
 179 (François et al. 2009); therefore:

$$\frac{\partial(\rho_l)}{\partial t} = \rho_l \beta_l \frac{\partial u_l}{\partial t} - \rho_l \beta'_l \frac{\partial T}{\partial t} \quad (6)$$

180 where β_l is the compressibility coefficient of liquid water and β'_l is the thermal expansion
 181 coefficient of water.

182 The solid grains are assumed to be incompressible by stress. Therefore the density of the
 183 solid phase is expressed only as a function of temperature:

$$\frac{\partial(\rho_s)}{\partial t} = -\rho_s \beta'_s \frac{\partial T}{\partial t} \quad (7)$$

184 where β'_s is the thermal expansion coefficient of solid particles.

185 Replacing equations (4) to (7) into equation (3) and rearranging the similar terms, yields:

$$\rho_l [n\beta_l + m_v] \frac{\partial u_l}{\partial t} + \rho_l [n(\beta'_s - \beta'_l) + \alpha] \frac{\partial T}{\partial t} - \rho_l m_v \frac{\partial \sigma}{\partial t} = -\nabla \cdot (\rho_l \mathbf{v}_l) \quad (8)$$

186 The mechanisms of water flow are considered to be due hydraulic, thermo-osmosis and
 187 gravitational potentials (Gonçalvès and Trémosa 2010). It is noted that the chemical osmosis
 188 is not considered here. Using Darcy's law and considering the potentials for water flow
 189 described above, the velocity of liquid can be presented as (Gonçalvès and Trémosa 2010):

$$\mathbf{v}_l = -k_l \left(\frac{\nabla u_l}{\rho_l g} + \frac{k_T}{k_l} \nabla T + \nabla z \right) \quad (9)$$

190 where k_l is the saturated hydraulic conductivity, k_T stands for the thermo-osmotic
 191 conductivity, z is the elevation and g is the gravitational constant.

192 The general form of water flow described in equation (8) can be extended considering the
 193 water flow mechanisms presented in equation (9) as:

$$\begin{aligned} \rho_l[n\beta_l + m_v] \frac{\partial u_l}{\partial t} + \rho_l[n(\beta'_s - \beta'_l) + \alpha] \frac{\partial T}{\partial t} - \rho_l m_v \frac{\partial \sigma}{\partial t} \\ = \nabla \cdot \left(\frac{k_l}{g} \nabla u_l \right) + \nabla \cdot (k_T \rho_l \nabla T) + \nabla \cdot (k_l \rho_l \nabla z) \end{aligned} \quad (10)$$

194 Equation (10) can be further simplified under the constant total stress as:

$$\begin{aligned} \rho_l[n\beta_l + m_v] \frac{\partial u_l}{\partial t} + \rho_l[n(\beta'_s - \beta'_l) + \alpha] \frac{\partial T}{\partial t} \\ = \nabla \cdot \left(\frac{k_l}{g} \nabla u_l \right) + \nabla \cdot (k_T \rho_l \nabla T) + \nabla \cdot (k_l \rho_l \nabla z) \end{aligned} \quad (11)$$

195 **Thermal behaviour**

196 The governing equation of heat transfer has been developed based on the energy conservation
 197 law in unsaturated porous media (Thomas and He 1997). Based on the formulation presented
 198 in Thomas and He (1997), the governing equation of heat transfer in saturated soil can be
 199 presented as:

$$\frac{\partial}{\partial t} [H_c(T - T_r)] = -\delta V \nabla \cdot [-\lambda_T \nabla T + A(T - T_r)] \quad (12)$$

200 where H_c is the heat storage capacity, T_r represents the reference temperature, λ_T is the
 201 thermal conductivity and A stands for the sum of the heat convection components.

202 Details of the expanded formulation for the governing equations of heat transfer presented in
 203 equation (12) can be found in Thomas and He (1997).

204 **Numerical model development**

205 The formulation of water flow is implemented within an existing numerical model
206 (COMPASS) developed at the Geoenvironmental Research Centre, Cardiff University which
207 is based on finite element and finite difference methods (Thomas and He 1997; Thomas et al.
208 2012). The Galerkin weighted residual method has been adopted by which the spatial
209 discretisation is developed and the temporal discretisation is achieved by applying an implicit
210 finite difference algorithm (Thomas and He 1997; Seetharam et al. 2011; Sedighi et al. 2016).
211 The model has been extensively tested, applied and extended to study the coupled behaviour
212 of unsaturated soils, in particular, the behaviour of highly compacted swelling clays in
213 relation to geological disposal of high level radioactive waste (e.g. Thomas et al. 2003; Cleall
214 et al. 2007; Vardon et al. 2011; Thomas and Sedighi 2012). The chemistry of clay-water
215 interaction and the effects on coupled processes of highly compacted swelling clays have
216 recently been integrated in the model (Sedighi 2011; Sedighi and Thomas 2014). Details of
217 the numerical formulation and computational aspects have been discussed in previous
218 publications; therefore the details are not repeated here.

219 The components of the hydraulic behaviour considering thermo-osmosis, thermal expansion
220 relationships and associated parameters of consolidation presented in equation (11) are
221 implemented in the numerical model. It is noted that the application of the model presented in
222 this paper is under constant total stress. Therefore the simulations have been carried using the
223 thermo-hydraulic modules of the model.

224

225 **Verification under steady state conditions**

226 Inclusion of the thermo-osmosis formulation in the water flow equation (equation 11) in the
227 numerical model is tested here. The problem considered is a fully saturated clay where a

228 fixed temperature gradient in the system causes a gradient of water pressure. As the flow
 229 occurs horizontally, the gravitational effects can be neglected. Under these conditions, the
 230 temporal variations of temperature and pore water pressure become zero and equation (11)
 231 can be simplified as:

$$\frac{k_l}{g} \nabla u_l = -k_T \rho_l \nabla T \quad (13)$$

232 This section presents the results of a verification test on the inclusion of thermo-osmosis
 233 component for moisture flow in comparison with the above algebraic equation and under the
 234 same conditions. A two dimensional soil domain (1.0 × 0.1 m) is considered in this
 235 simulation. The soil is assumed to be fully saturated. Temperatures at the boundaries are
 236 assumed to be fixed values, i.e. 300K and 285K which provide a constant temperature
 237 gradient over the domain at steady-state condition. The water pressure is considered fixed at
 238 the boundary with lower temperature while the other side of the domain is considered as an
 239 impermeable boundary for water flow. The initial pore water pressure is assumed to be equal
 240 to the water pressure at the boundary, i.e. 100 kPa.

241 The domain is discretised into 100 equally sized 4-noded quadrilateral elements. A constant
 242 time-step of 3600 seconds is considered. Material parameters used in the simulation are
 243 presented in Table 1.

244 By knowing the temperature gradient in the system, the gradient of water pressure can be
 245 analytically calculated at steady state using equation (13). For the conditions and parameters
 246 used in this problem, the gradient of pore water pressure can be given as:

$$\nabla u_l = - \frac{5.0 \times 10^{-12} \times 998.0 \times 9.81}{1.0 \times 10^{-11}} \times \frac{285 - 300}{1} = 73427.85 \text{ Pa. m}^{-1}$$

247 The pore water pressure at the left hand side boundary with temperature of 300K is therefore
248 obtained to be equal to 26572.15 Pa.

249 Fig. 1 presents the profile of water presurre in the domain. The same value for the gradient of
250 pore water pressure is obtained from the numerical results as that calculated from the
251 analytical solution. It can be observed that the pore pressure decreases in the warmer
252 boundary of the domain due to the thermo-osmosis effect in order to maintain the overall
253 water potential balance in the system.

254

255 **Pore water pressure development in a heating experiment**

256 In order to examine the effects of thermo-osmosis in pore water pressure development under
257 non-isothermal conditions, numerical simulations are carried out based on a field scale
258 heating experiment (ATLAS test) carried out in Mol, Belgium in an underground research
259 facility at a depth of 223 m in Boom Clay (François et al. 2009; Chen et al. 2011). The
260 simulation conditions including the domain, initial conditions and boundary conditions are
261 selected based on the experimental conditions described in François et al. (2009). The
262 numerical analysis includes studying the pore water pressure generation due to thermal load
263 through two series of analysis: i) without considering thermo-osmosis in the analysis and ii)
264 by including the thermo-osmosis effects.

265 The simulation problem is a 100 m axisymmetric domain where the inner boundary is
266 impermeable for flow. At the inner boundary, temperature is instantaneously increased by
267 45K from the initial value of 289.5K and kept constant for 3 years. Temperature is then
268 increased by 40K and kept constant for 11 months after which it is restored back to the initial
269 temperature and remains constant until the end of the simulation. At the outer boundary, the

270 temperature and the pore water pressure are fixed at the same value of the initial condition,
271 i.e. 289.5K and 2.025 MPa, respectively.

272 The physical properties of Boom Clay have been presented in several published works (e.g.
273 Baldi et al. 1988; Hong et al. 2013; Hueckel and Baldi 1990; Sultan et al. 2002; Cui et al.
274 2009; François et al. 2009; Chen et al. 2011). A summary of the material properties used in
275 this study is presented in Table 2. For the sake of simplicity, the outlined parameters are
276 taken as constants in the numerical simulation. The value of thermo-osmotic permeability is
277 calculated based on data presented in Gonçalves and Trémosa (2010) and Gonçalves et al.
278 (2012) who provided a ratio of osmotic permeability to intrinsic permeability (k), i.e. k_T/k
279 as a function of half-pore thickness of the porous media. Using the specific surface area
280 values ranging between $150 \text{ m}^2.\text{g}^{-1}$ and $163 \text{ m}^2.\text{g}^{-1}$ presented in Deng et al. (2011) and half-
281 pore thickness equation in Gonçalves et al. (2012), a value for half-pore thickness of
282 approximately 1.5nm for the Boom clay is calculated. Hence, following Gonçalves and
283 Trémosa (2010) and Gonçalves et al. (2012), an approximate value for thermo-osmotic
284 conductivity of $3 \times 10^{-12} \text{ m}^2.\text{K}^{-1}.\text{s}^{-1}$ is chosen. It is noted that in the absence of exact values for
285 thermo-osmotic conductivity, the approximated value may involve a level of uncertainty, yet
286 providing a reasonable value to be used for investigating the effect of thermo-osmosis
287 phenomenon on pore water pressure development in this example.

288 The domain is discretised to 300 unequally sized 4-nodded quadrilateral elements. A
289 maximum time-step of one week is considered for the simulations for a total period of 2,500
290 days.

291 Fig. 2 presents the results of temperature variation with time at 1.183 m distance from the
292 symmetrical axis of the domain. The results are compared with those observed in the

293 experiment (François et al. 2009). From Fig. 2, it can be observed that the modelling results
294 are in close agreement with the experimental data. The numerical results show a slight
295 overestimation of the temperature evolution during the stages of temperature increase, while
296 the numerical results agree well with the experimental data at the stages where temperature
297 has been restored back to the initial soil temperature.

298 The results of the pore water pressure development with time at 1.183 m distance from
299 symmetrical axis are presented in Fig. 3. Experimental data are also shown in Fig. 3. Two
300 series of results from the numerical analysis are presented including the results of modelling
301 without thermo-osmosis effects and those with this effect. The pore water pressure
302 development in the analysis without thermo-osmosis is only related to thermal expansion
303 effect. The numerical trend obtained is similar to the observation in the experiment. However,
304 there are considerable discrepancies in terms of pore water dissipation at the end of both
305 heating phases when temperature stabilised. In other words, the simulation without thermo-
306 osmosis effect over-predicts the pore water pressure development. It can be observed that
307 closer agreements with the experimental data are achieved when combined effects of thermo-
308 osmosis and thermal expansion are considered in the simulations. The phenomenon can be
309 explained by analysing the effect of thermo-osmosis on the fluid flow. Water flow is
310 diverging from the warmer side towards the colder side of the domain causing a pressure
311 decrease closer to the heat source. From the results presented in Fig. 3, it can be observed that
312 during the stage at which temperature is stable, the thermo-osmosis contributes to the pore
313 water pressure decrease. A similar observation has been presented in the work of Trémosa et
314 al. (2010) where after an applied temperature pulse, the contribution of thermo-osmosis to the
315 pore water pressure dissipation was found to be more pronounced than the reduction of the
316 water volume due to the temperature decrease.

317 The results of the numerical model show good agreement with the first pore water dissipation
318 stage while the results for the second and the third dissipation stages show slightly different
319 pressure values compared with the experimental data. However, the general trend
320 demonstrates a similar qualitative trend observed in the experiment. The discrepancies
321 observed both for temperature and pore pressure evolutions may be related to various factors
322 including the heat diffusion and liquid flow via radial direction (axisymmetric analysis), the
323 approximated value for thermo-osmotic conductivity, duration of the simulation and constant
324 values considered for the parameters used in the numerical simulation.

325

326 **Simulation of thermo-osmosis effects**

327 **Numerical analysis**

328 This section provides the results of a series of numerical simulations on the hydraulic
329 behaviour of clay around a heating source. The aim of the simulations is to investigate the
330 effects of thermo-osmosis on temporal and spatial pore water pressure developments around a
331 heating source in saturated clay.

332 The problem studied is a two dimensional saturated soil domain (30 m × 1 m) which is heated
333 at one boundary by a heating source 0.6 m in diameter. An axisymmetric analysis is
334 considered where the axis of the symmetry is at the centre of the heat source. The internal
335 boundary of the model corresponds to the external boundary of the heat source, which is 0.3
336 m away from the axis of the symmetry. The external boundary of the model is placed at 30.3
337 m from the symmetrical axis of the domain. The inner boundary condition is considered as an
338 impermeable and adiabatic where a time curve is applied for temperature values.
339 Temperature is constantly increasing from an initial value of 285K for the first 2 days until it

340 reaches a fixed value at 310K and then remains constant throughout the rest of the
341 computation. At the external boundary, temperature and the pore water pressure are fixed to
342 be the same as the initial condition values of 285K and 100 kPa, respectively.

343 The domain is discretised to 300 equally sized 4-noded quadrilateral elements. A maximum
344 time-step of 2 hours (7200 seconds) is considered and the duration of the simulation is 30
345 days.

346 The soil parameters used in the simulations are summarised in Table 3. The simulation is
347 carried out using three values of thermo-osmotic conductivity based on the range of (k_T/k)
348 provided by Gonçalves et al. (2012) given as $k_T/k = 10$, $k_T/k = 100$ and $k_T/k = 1000$.
349 The variation of water viscosity with temperature is not considered following the approach
350 presented in Soler (2001) and Ghassemi and Diek (2002) and a constant value of 10^{-3} Pa.s at
351 20 °C is selected.

352 It should be noted that the thermo-osmotic conductivity is calculated with respect to intrinsic
353 permeability with a value of $3.3 \times 10^{-17} \text{ m}^2$. As described by Goncalves et al. (2012), this ratio
354 is expressed in Pa.K^{-1} which means that the intrinsic permeability has to be divided by the
355 viscosity first in order to obtain the value of k_T in $\text{m}^2.\text{K}^{-1}.\text{s}^{-1}$. Therefore, water viscosity is
356 taken as a constant value at 20°C.

357 The compressibility of the studied soils is calculated based on the specific surface area
358 yielding an approximate value of coefficient of soil volume compressibility. A reference
359 value of the specific surface area of $30 \text{ m}^2.\text{g}^{-1}$ is taken (Goncalves et al. 2012). Using
360 approach proposed in Mitchell (1993) where an empirical relation between liquid limit and
361 specific surface is given, a liquid limit is obtained through which an approximate value of
362 coefficient of consolidation is assumed. Hence, using the obtained value for coefficient of

363 consolidation and known value of hydraulic conductivity, an approximate value of coefficient
364 of soil volume compressibility of $3.3 \times 10^{-8} \text{ Pa}^{-1}$ is calculated and used in the simulation which
365 falls within a range of compressibility values for medium-hard clays.

366 The results of simulations using different coefficients of thermo-osmotic conductivity are
367 presented and compared with the case where only thermal expansion is considered. Fig. 4
368 presents the variation of pore water pressure with time at the heat source and soil interface
369 boundary. It can be observed that for the lowest value of thermo-osmotic conductivity used,
370 i.e. $3.4 \times 10^{-13} \text{ m}^2 \cdot \text{K}^{-1} \cdot \text{s}^{-1}$, the difference between the pore water pressure development in
371 comparison with the case where only thermal expansion was considered is negligible, while
372 for the higher values of thermo-osmotic conductivity the difference is more highlighted. As it
373 can be observed after 5 days of heating, the pore water pressure value at the interface is
374 14.1% less when $k_T = 3.4 \times 10^{-11} \text{ m}^2 \cdot \text{K}^{-1} \cdot \text{s}^{-1}$ is used, compared to the analysis without thermo-
375 osmosis contribution.

376 Due to the liquid flow caused by the temperature gradient in the domain, it can be expected
377 that the rate of pressure drop would only enhance with time. This can be confirmed from Fig.
378 4 where after 25 days, pressure drop caused by thermo-osmosis increases to 16.31% for the
379 case of $k_T = 3.4 \times 10^{-11} \text{ m}^2 \cdot \text{K}^{-1} \cdot \text{s}^{-1}$ in comparison with the results of simulations with thermal
380 expansion only.

381 Temperature profiles in the domain after 2, 7 and 30 days are presented in Fig. 5. After 30
382 days of heating at a constant temperature of 310 K, heat propagates up to a distance of
383 approximately 3.5 m from the heat source creating a temperature gradient in the domain.

384 Further discussion on the spatial variation of pore pressure development in the domain is
385 presented for the simulation with high thermo-osmotic conductivity of $3.4 \times 10^{-11} \text{ m}^2 \cdot \text{K}^{-1} \cdot \text{s}^{-1}$. In

386 addition, the results are compared with the case where only thermal expansion is included in
387 the simulation. Profiles of the pore water pressure in the domain for the case of the thermo-
388 osmotic conductivity of $3.4 \times 10^{-11} \text{ m}^2 \cdot \text{K}^{-1} \cdot \text{s}^{-1}$ are shown in Fig. 6. The results are presented for
389 three different heating periods, i.e. 2 days, 7 days and 30 days after the start of the simulation.
390 The difference in pore water pressure evolution between the cases without and with the effect
391 of thermo-osmosis is considerable as shown in Fig 6. It can be observed that after 2 days of
392 heating, thermo-osmosis affects the pore water pressure evolution next to the heat source
393 yielding a reduction in the pore water pressure value for 12% in comparison to the case with
394 thermal expansion only. Due to the temperature gradient established in the vicinity of a heat
395 source, the effect of thermo-osmosis becomes more pronounced with an increase in
396 simulation time yielding a reduction in pore water pressure of 15% and 17% after 7 and 30
397 days, respectively. In addition, it can be seen that the pore water pressure almost returns to
398 the initial value at the interface after 30 days of heating due to thermo-osmosis effect. As a
399 consequence of water flowing away from the heat source under the temperature gradient, the
400 water pressure is slowly increasing further away in the domain.

401 **The effects of thermo-osmosis**

402 The results demonstrate the relative importance of thermally driven flow on pore water
403 pressure in the case of high thermo-osmotic properties. Under the conditions of the
404 simulations presented, it can be concluded that values of thermo-osmotic conductivity larger
405 than $10^{-12} \text{ m}^2 \cdot \text{K}^{-1} \cdot \text{s}^{-1}$ can affect the water pressure field around the heat source. This
406 conclusion is in agreement with the observation reported in Soler (2001) who has performed
407 simple one-dimensional transport simulations including thermal and chemical osmosis,
408 hyper-filtration and thermal diffusion with the objective of estimating the effects of different

409 coupled transport phenomena where a temperature gradient of 0.25 K.m^{-1} has been used.
410 Author has concluded that thermo-osmosis will have a significant effect if the value of
411 thermo-osmotic conductivity is larger than $10^{-12} \text{ m}^2.\text{K}^{-1}.\text{s}^{-1}$.

412 As previously mentioned, values of the ratio of thermo-osmotic conductivity and intrinsic
413 permeability against the total ionic content used in this paper are adopted from the data
414 presented by Gonçalves and Trémosa (2010) and Gonçalves et al. (2012). This approach
415 adopted provides relationships between the thermo-osmotic conductivity and clay surface-
416 charge, bulk fluid concentration, type of counter ion in the pore water and pore size. In
417 addition, authors have verified it with the available data which ones again confirm the
418 electrochemical control of thermo-osmosis. Hence, for a constant value of intrinsic
419 permeability, coefficient of thermo-osmotic conductivity could be influenced and modified
420 by a combination of the aforementioned soil properties.

421 It is noted that the boundary conditions can have considerable influence on the pore water
422 pressure evolution. In the case study presented, the outer boundary was considered to be a
423 constant pore water pressure representing the far field condition. Under other scenarios where
424 the outer boundary is impermeable or several heat sources are located close to each other, the
425 thermo-osmosis could enhance the pore water pressure and induce local reduction in the
426 effective stress. This could be an important aspect as the reduction in the effective stress
427 could cause deformation and geomechanical instability.

428 Due to the fact that the temperature gradient is the driving potential for thermo-osmosis, the
429 observed phenomenon could be further highlighted in media with low thermal conductivity
430 where a large temperature gradient can develop next to the heat source. Using a value of
431 $3.4 \times 10^{-10} \text{ m.s}^{-1}$ for hydraulic conductivity in this study, it was observed that water flow both

432 under pressure gradients and temperature gradients contributes to the overall flow of water in
433 the system. However, in saturated porous media with hydraulic conductivity lower than the
434 abovementioned, liquid flow due to thermo-osmosis could become a dominant process
435 prevailing over a classical Darcian flow. This could be important in the context of a nuclear
436 waste disposal and radionuclide release where high temperature gradient is established in the
437 vicinity of waste packages and thermo-osmosis might contribute to the overall convective
438 transport of water and radionuclides. As this simulation for 30 days of heat emission shows
439 that thermo-osmosis affects the pore water pressure field around the heat source only when
440 relatively high value of thermo-osmotic conductivity is considered, this might not be the case
441 where heat emission lasts for prolonged time periods, i.e. thousands of years for the case of
442 nuclear waste. In such case, it can be expected that even lower values of thermo-osmotic
443 conductivity might contribute to the water flow in low permeability porous media. For such
444 purpose, further research should be undertaken for reliable estimation of the thermo-osmotic
445 conductivity value and its dependence on temperature and soil properties.

446

447 **Conclusions**

448 The work presented describes the effects of thermo-osmosis phenomenon on hydraulic
449 behaviour of fully saturated soils. The theoretical formulation accommodates thermo-osmosis
450 in the formulation of water flow under coupled thermal, hydraulic and mechanical behaviour.
451 The developed model was tested against an analytical benchmark and a heating experiment in
452 order to examine the accuracy of the model development and implementation. It was shown
453 that the inclusion of thermo-osmosis in the coupled thermo-hydraulic simulation of a heating

454 experiment provides a better agreement with the experimental data compared with the case
455 where only thermal expansion of the soil constituents was considered.

456 A series of simulations were performed that examine the pore water pressure behaviour under
457 heating. Through the numerical simulations presented, it was shown that the thermally driven
458 liquid water flow due to thermo-osmosis can affect the pore water pressure evolution in the
459 vicinity of the heat source. Under the conditions of the simulation related to the clay
460 behaviour subjected to a constant heating source, it was found that the effect of thermo-
461 osmosis is considerable at thermo-osmotic conductivity values larger than $10^{-12} \text{ m}^2 \cdot \text{K}^{-1} \cdot \text{s}^{-1}$.

462

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467

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590 thermo-osmosis ($k_T = 3.4 \times 10^{-11} \text{ m}^2 \cdot \text{K}^{-1} \cdot \text{s}^{-1}$)

591

592

Table 1. Material parameters for verification exercise

Material parameters	Value
Porosity	0.4
Water density	998.0 kg.m ⁻³
Hydraulic conductivity	1.0×10 ⁻¹¹ m.s ⁻¹
Thermo-osmotic conductivity	5.0×10 ⁻¹² m ² .K ⁻¹ .s ⁻¹

593

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Table 2. Material parameters for validation exercise

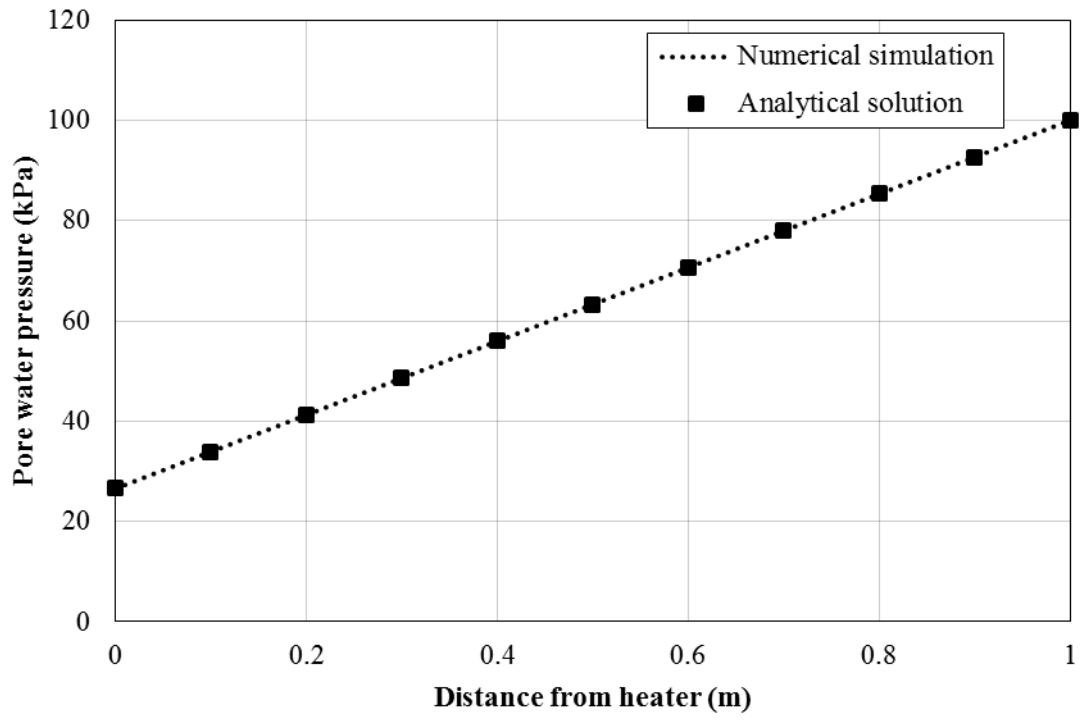
Material Parameters	Value	Reference
Porosity	0.4	François et al. (2009)
Solid density	2670.0 kg.m ⁻³	François et al. (2009)
Specific heat capacity of liquid	4186.0 J.kg ⁻¹ .K ⁻¹	François et al. (2009)
Specific heat capacity of solid	732.0 J.kg ⁻¹ .K ⁻¹	François et al. (2009)
Thermo-osmotic conductivity	3.0x10 ⁻¹² m ² .K ⁻¹ .s ⁻¹	Gonçalvès et al. (2012)
Thermal conductivity	1.69 W.m ⁻¹ .K ⁻¹	François et al. (2009)
Hydraulic conductivity	1.5x10 ⁻¹² m.s ⁻¹	François et al. (2009)
Solid thermal expansion coefficient	1.0x10 ⁻⁵ K ⁻¹	François et al. (2009)
Water thermal expansion coefficient	3.5x10 ⁻⁴ K ⁻¹	François et al. (2009)
Water compressibility	4.5x10 ⁻¹⁰ Pa ⁻¹	François et al. (2009)
Soil compressibility	4x10 ⁻⁹ Pa ⁻¹	François et al. (2009)
Soil thermal expansion coefficient	-5.0x10 ⁻⁵ K ⁻¹	Hong et al. (2013)

Table 3. Material parameters for thermo-osmosis simulation

Material parameters	Value
Porosity	0.47
Solid density	2630.0 kg.m ⁻³
Specific heat capacity of liquid	4186.0 J.kg ⁻¹ .K ⁻¹
Specific heat capacity of solid	937.0 J.kg ⁻¹ .K ⁻¹
Thermo-osmotic conductivity	(1) 3.4×10 ⁻¹¹ m ² .K ⁻¹ .s ⁻¹
	(2) 3.4×10 ⁻¹² m ² .K ⁻¹ .s ⁻¹
	(3) 3.4×10 ⁻¹³ m ² .K ⁻¹ .s ⁻¹
Thermal conductivity	1.55 W.m ⁻¹ .K ⁻¹
Hydraulic conductivity	3.4×10 ⁻¹⁰ m.s ⁻¹
Solid thermal expansion coefficient	3.0×10 ⁻⁵ K ⁻¹
Water thermal expansion coefficient	3.5×10 ⁻⁴ K ⁻¹
Water compressibility	4.5×10 ⁻¹⁰ Pa ⁻¹
Soil compressibility	3.3×10 ⁻⁸ Pa ⁻¹
Soil thermal expansion coefficient	-5.0×10 ⁻⁵ K ⁻¹

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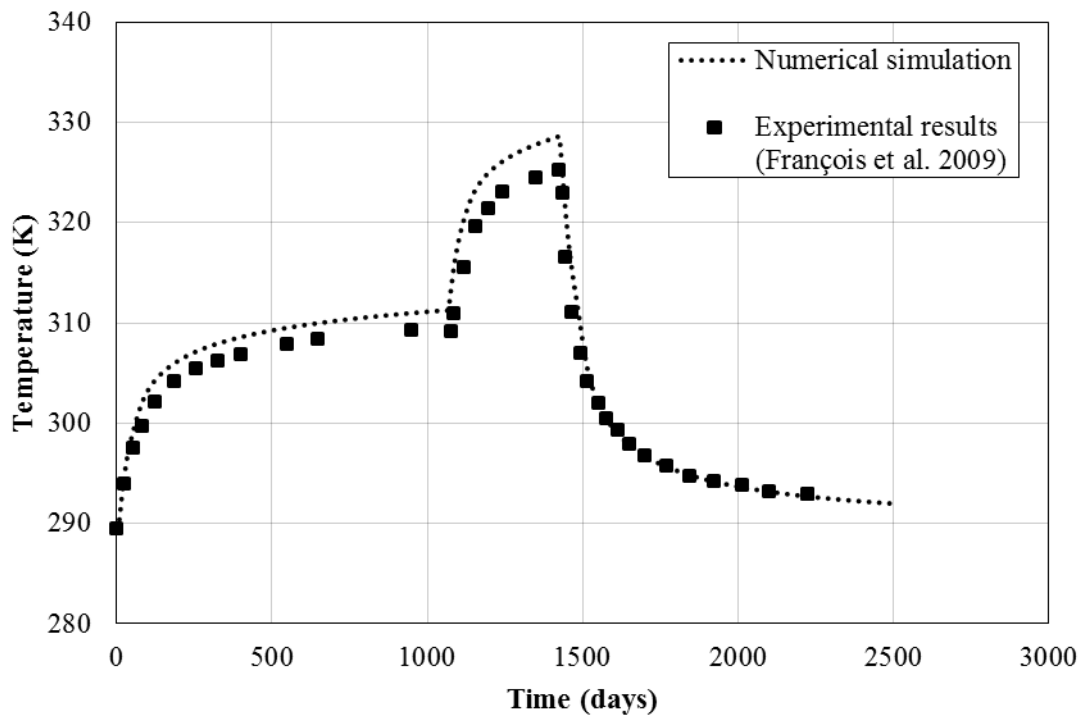


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Fig. 1. Profile of pore water pressure in the domain at steady-state

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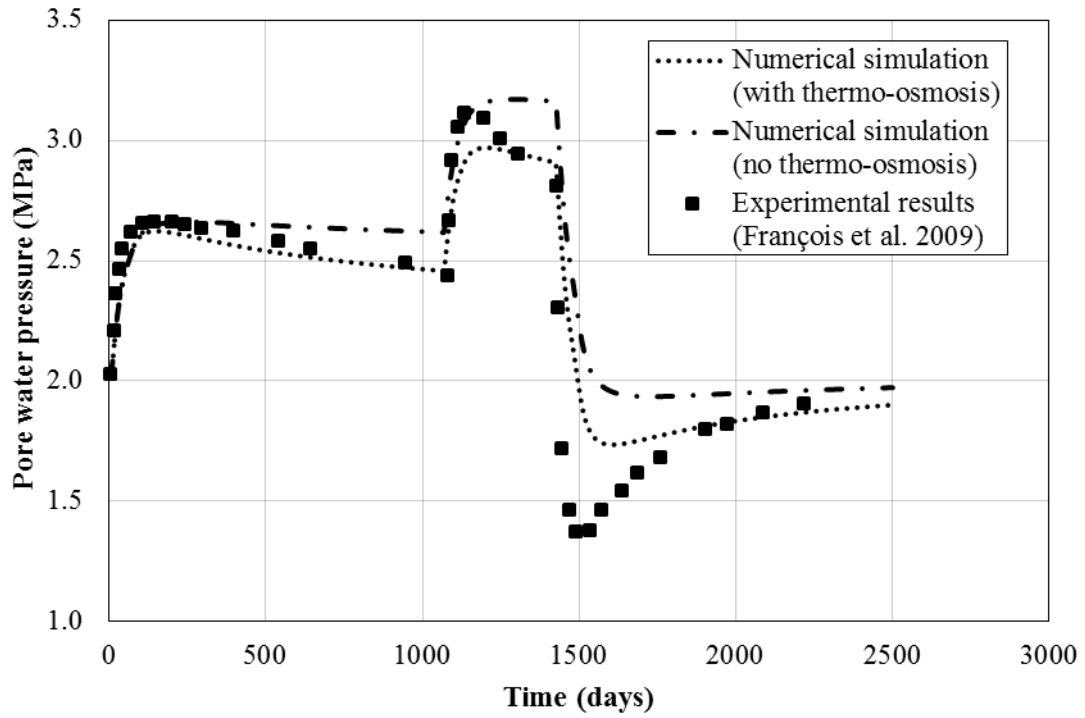
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605 Fig. 2. Variation of temperature with time at the distance of 1.183 m from the axis of

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symmetry

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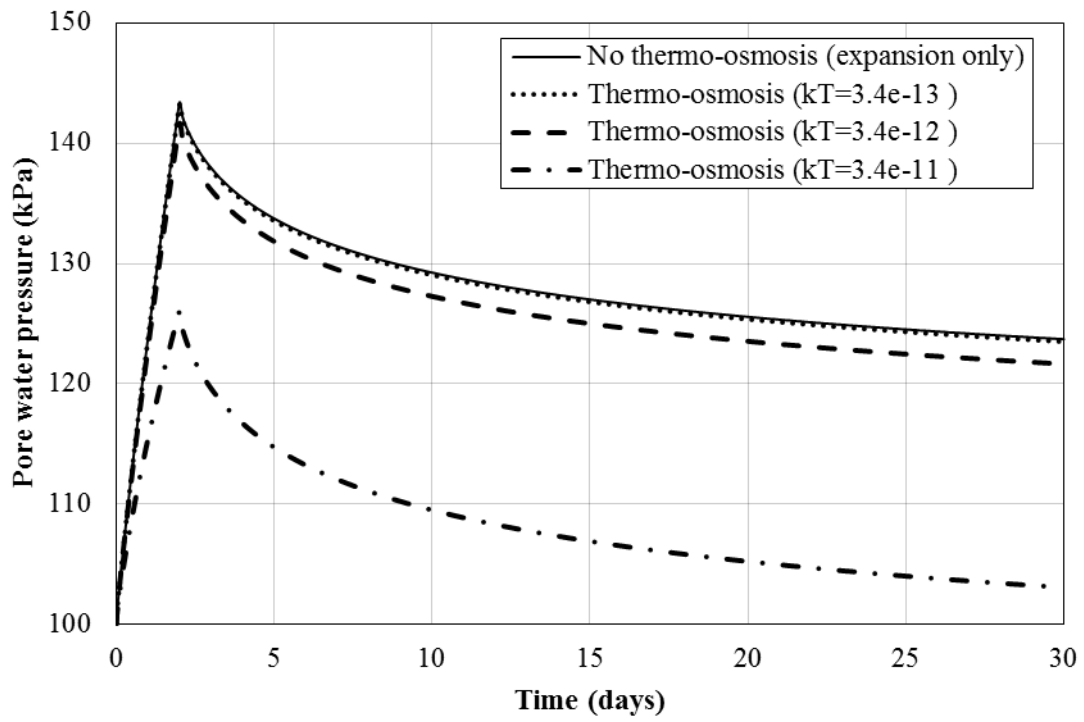
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609 Fig. 3. Variation of pore water pressure with time at the distance of 1.183 m from the axis of

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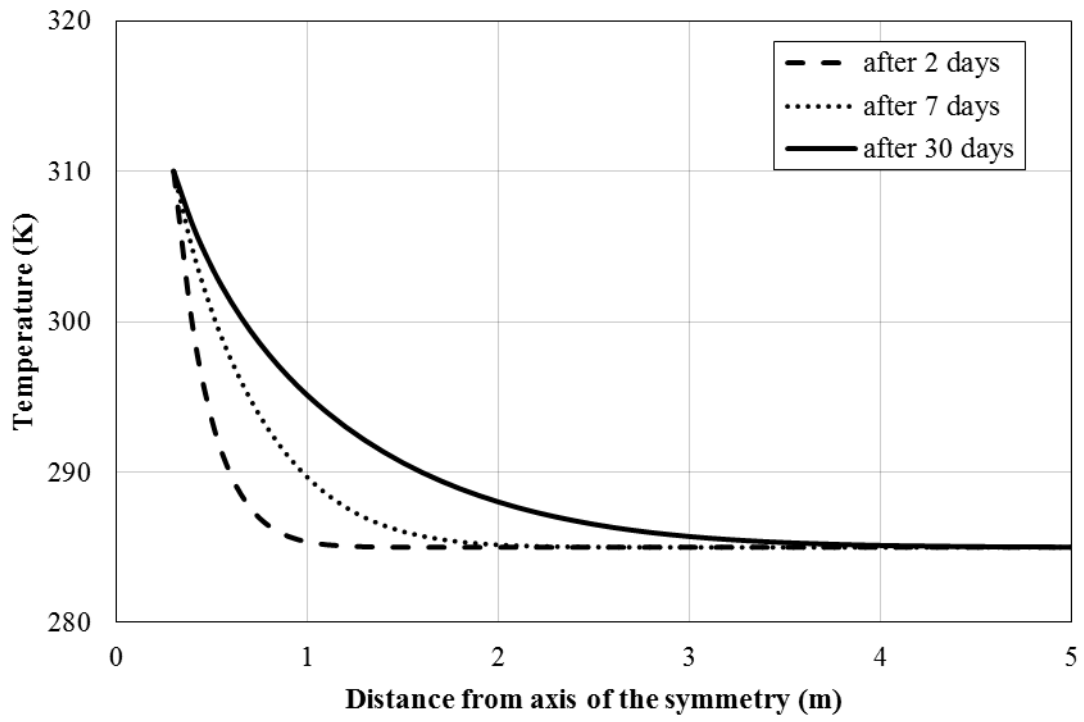
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613 Fig. 4. Pore water pressure evolution with time at the heat source-soil interface

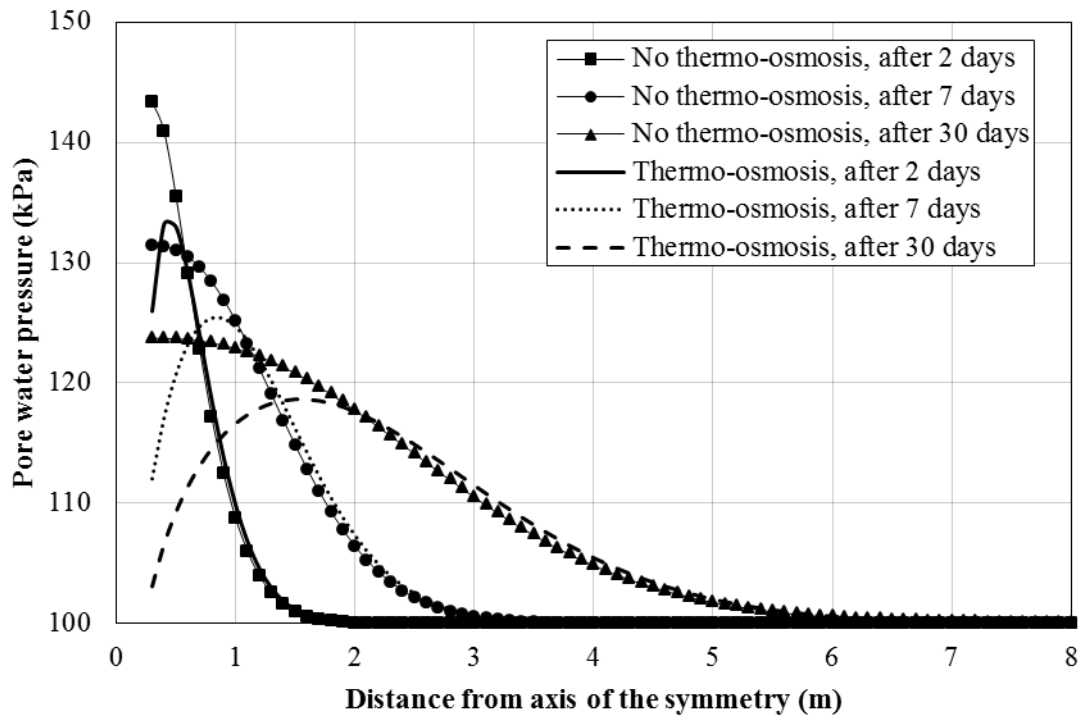
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616 Fig. 5. Temperature distributions around the heat source after 2, 7 and 30 days of heating

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619 Fig. 6. Pore water pressure variation in the domain for the case of analysis without and with

620 thermo-osmosis ($k_T = 3.4 \times 10^{-11} \text{ m}^2 \cdot \text{K}^{-1} \cdot \text{s}^{-1}$)

621