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Title: Leaching and Microstructural Properties of Lead Contaminated
Kaolin Stabilized by GGBS-MgO in Semi-Dynamic Leaching Tests

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Abstract: Ground granulated blast furnace slag (GGBS) is widely used to stabilize soils due to its environmental and economic merits. The strength and durability of reactive MgO activated GGBS (GGBS-MgO) stabilized lead (Pb)-contaminated soils have been explored by previous studies. However, the effects of simulated acid rain (SAR) on the leachability and micro-properties of GGBS-MgO stabilized Pb-contaminated soils are hardly investigated. This research studies the leachability and microstructural properties of GGBS-MgO stabilized Pb-contaminated kaolin clay exposed to SAR with initial pH values of 2.0, 4.0 and 7.0. A series of tests are performed including the semi-dynamic leaching tests using SAR as the extraction liquid, acid neutralization capacity (ANC), mercury intrusion porosimetry (MIP), and X-ray diffraction (XRD) tests. The results demonstrate that as the SAR pH decreases from 7.0 to 4.0, the cumulative fraction leached (CFL) and observed diffusion coefficient (Dobs) of Pb increases significantly. Meanwhile, increasing the GGBS-MgO content from 12% to 18% results in decrease of CFL and Dobs. Further decreasing the SAR pH to 2.0 results in the dissolution-controlled leaching mechanism and more notable increase in CFL regardless of the binder dosage. The differences in the leaching properties under different pH conditions are interpreted based on the cemented soil acid buffering capacity, hydration products and pore size distributions obtained from the ANC, MIP, and XRD tests, respectively.

Research Highlights:

- Leaching is diffusion-controlled at pH 7 and 4 while dissolution-controlled at pH 2
- Observed diffusion coefficient is affected by the SAR pH and binder dosage
- Pb is primarily precipitated as hydrocerussite ($\text{Pb}_2(\text{CO}_3)_2(\text{OH})_2$) in soil matrix
- ANC and pore structure of the treated soil affect diffusive properties of Pb

1 **Leaching and Microstructural Properties of Lead Contaminated**
2 **Kaolin Stabilized by GGBS-MgO in Semi-Dynamic Leaching Tests**

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1 Introduction

Numerous abandoned industrial sites worldwide have been found to be contaminated with a wide range of heavy metals [1-7]. These toxic metals such as lead (Pb), zinc (Zn), copper (Cu), and cadmium (Cd), if treated improperly, can pose severe threats to the environment and human health. Considering the fast urbanization and ever-increasing value of the land resources, particularly in the developing countries such as China and India, it is imperative to develop effective and economical technologies to remediate these heavy metal contaminated industrial sites. The ultimate goal is to eliminate their negative environmental impact to the society and improve the mechanical properties of soils to facilitate post-construction. Solidification/Stabilization (S/S) has been widely used to immobilize contaminants and improve the soil properties [2-3, 8]. After S/S, the remediated soils can be reused in-situ as engineering construction materials, which would help on the fast redevelopment of the contaminated site [9-10].

Portland cement (PC) is the most popular binder used in S/S [11]. However, its manufacturing process is associated with high power consumption (5000 MJ/t PC), non-renewable resources usage (1.5 t limestone and clay/t PC) and considerable emissions of carbon dioxide (CO₂), dust, and deleterious gases (SO₂, CO, NO_x) (0.95 t/t PC) [12-13]. Therefore, full or partial replacement of PC by more sustainable industrial by-products (e.g. fly ash and slag) as alternative binders in treating contaminated soil has received ever-increasing popularity. One of the promising alternative binders is alkali-activated slag (AAS) cement using ground granulated

blast furnace slag (GGBS) as the main raw material. However, several drawbacks are associated with the utilization of AAS in S/S including over-rapid setting, difficulty in handing/transporting the caustic alkalis and uneconomical efficiency [12]. To address these issues, reactive magnesia (MgO) has been used as an effective activator for the GGBS [12, 14-16]. Existing studies on the GGBS-MgO binder mainly focus on the strength, durability and microstructural properties of the pastes and stabilized soils [14-16]. The MgO facilitates the breakage of Si-O and Al-O bonds in the GGBS to promote the formation of the calcium silicate hydrate (C-S-H) and hydrotalcite ($\text{Mg}_6\text{Al}_2\text{CO}_3(\text{OH})_{16}$)-like phase (Ht) as the main hydration products [17-19] while C-S-H and $\text{Ca}(\text{OH})_2$ are the main hydration products in PC stabilized soils [2,21]. The C-S-H and Ht formed would enhance the physical and mechanical properties [14-16, 18] and reduce the leachability of contaminants in heavy metal contaminated soils [16, 20]. Recently, the feasibility of using this binder for stabilizing heavy metal-contaminated soils has been demonstrated both in the laboratory [20] and a field trial [8]. However, to date, no systematic studies exist on the diffusive properties of heavy metals in GGBS-MgO stabilized heavy metal contaminated soils.

Sharma and Reddy [6] indicated that the acid rain may vary from a highly acidic condition ($\text{pH} = 2.0$) to a neutral condition ($\text{pH} = 7.0$). It is reported that the average pH value of the acid rain in Nanjing City, China is about 5.09 with the lowest pH of 2.89 [21-23]. Du et al. [2] and Yun et al. [24] studied the leaching behavior and long-term durability of PC solidified/stabilized heavy metal-contaminated soils under various acid rain conditions. They showed that heavy metals could be released

notably from the stabilized soils with increased acidity. It is expected that due to the different hydration chemistry and reaction products formed in GGBS-MgO and PC binders, the leaching properties of the treated soils exposed to the acid rain would be different. Therefore, it is necessary to comprehensively evaluate the leaching behavior of GGBS-MgO stabilized heavy metal-contaminated soils under different acidic conditions: strongly acidic condition (pH = 2.0), moderate acidic condition (pH = 4.0) and neutral condition (pH = 7.0).

In this study, a series of semi-dynamic leaching tests are performed on lead (Pb)-contaminated kaolin clay using simulated acid rain as the extraction leachant with initial pH values of 2.0, 4.0, and 7.0. The effects of acid rain pH and GGBS-MgO content on the leachability and microstructural properties of the treated soils are studied. The semi-dynamic leaching test results are interpreted by acid neutralization capacity (ANC), mercury intrusion porosimetry (MIP) and X-ray diffraction (XRD). This study provides useful insights for remediating Pb-contaminated kaolin using the GGBS-MgO binder.

2 Materials and Testing Methods

2.1 Materials and sample preparations

Kaolin clay is used as a base soil due to its uniform composition (low organic content, homogeneity and uniform mineralogy) and low cation exchange capacity [1-3, 14]. The basic physiochemical properties of the kaolin clay are summarized in

Table 1. The pH is measured per [ASTM D4972 \[25\]](#) using a pH meter HORIBA D-54. The specific gravity is measured per [ASTM D5550 \[26\]](#). The Atterberg limits are measured per [ASTM D4318 \[27\]](#). The kaolin clay is classified as lean clay (CL) based on the Unified Soil Classification System [\[28\]](#). The moisture content is measured as per ASTM D2216 [\[29\]](#). The grain size distribution is measured using a laser particle size analyzer Mastersizer 2000.

The physiochemical properties of GGBS and MgO are listed in **Table 2**. The BET specific surface areas of the GGBS and MgO are measured by nitrogen adsorption using Physisorption Analyzer ASAP2020. The chemical compositions of the kaolin clay, GGBS, and MgO are measured using X-ray fluorescence (XRF) as shown in **Table 3**. The reactivity of the MgO is measured as the time duration required for the neutralization of an acidic solution (0.25 M acetic acid in this study) by a certain amount of MgO sample (5.0 g in this study) in which phenolphthalein is adopted as the pH indicator [\[30\]](#). The mean values of the above tests are presented in **Tables 1 to 3**.

Pb is used in this study because it is a very common toxic heavy metal in contaminated soils [\[3, 14, 31\]](#). Lead nitrate ($\text{Pb}(\text{NO}_3)_2$) powder (Chemical Analytical Reagent, Sinopharm Chemical Reagent Co., Ltd.) is dissolved in distilled deionized water (DDW) as stock solutions with predetermined Pb concentrations. The simulated acid rain (SAR), used as the extraction liquid (leachant) in the semi-dynamic leaching test, is prepared by diluting nitric acid (HNO_3) and ammonium sulfate ($(\text{NH}_4)_2\text{SO}_4$) in the DDW. Prior to adding HNO_3 , ammonium sulfate ($(\text{NH}_4)_2\text{SO}_4$) solution is added to

the DDW until the concentration of the sulfate ion (SO_4^{2-}) reaches 0.001 mol/L [2].

The stock solutions of SAR are adjusted to three pH value of 2.0, 4.0, and 7.0 respectively. SAR with pH of 2.0 represents a strong acid rain in the field [2].

Previous studies show that a binder with 9: 1 ratio of GGBS to MgO (dry weight basis) yields relatively higher strength and lower leachability of stabilized contaminated soils [14]. Therefore, the binder consisting of 90% GGBS and 10% MgO (dry weight basis) is prepared. Three binder contents are set as 12%, 15%, and 18% (dry weight soil basis) which are typical contents in engineering projects [2]. The water content and the Pb concentration are set as 45% and 2% (i.e., 20000 mg/kg) (dry weight soil basis) to simulate a heavily contaminated site soil [11, 21], respectively. Six mixtures are investigated in total and denoted as GM_iPb_j , where i = content of the GGBS-MgO binder (i.e., 12, 15 or 18), and j = Pb concentration (% , 0 or 2).

The kaolin clay, GGBS and MgO powders are placed in a plastic bottle and are thoroughly mixed by a bench-top mixer. Then the predetermined volume of $\text{Pb}(\text{NO}_3)_2$ stock solution is added to the plastic bottle and further mixed for 30 minutes by the mixer to sufficiently homogenize the mixture. The mixture is filled into a cylindrical PVC mold ($\Phi 50 \times H 100$ mm) in five equal height layers. The mold is vibrated manually after each filling to eliminate air bubbles. After five fillings, the mixture is cured under the standard condition ($20 \pm 2^\circ\text{C}$, relative humidity = 95%) for 28 days. In addition, the GGBS-MgO cement paste (GGBS : MgO = 9:1, water : cement = 0.6) is prepared following the same procedure but without adding kaolin clay and

Pb(NO₃)₂ solution. Totally six identical soil samples are prepared with four samples subjected to the semi-dynamic leaching test, and two samples used for the measurement of specific gravity, water content and density before and after the semi-dynamic leaching test. The crushed and sieved sample was also used for ANC and XRD tests. In addition, one GGBS-MgO paste sample is prepared for XRD test.

In the authors' previously studies [2, 14], contaminated soils were prepared by spiking clean soil with heavy metal solution at controlled water content, and cured under controlled condition ($20 \pm 2^\circ\text{C}$, relative humidity = 95%) until chemical equilibrium between soil and heavy metal is achieved. The mixture is then thoroughly mixed with binder with designed dosage, compacted under controlled dry density and water content, and cured before subjected to various tests. The soil sample preparation method presented in this study is more time effective but the chemical reaction between Pb and kaolin may not achieve equilibrium, which may influence the leaching characteristic and microstructural properties of the stabilized soils. Further study is warranted to address this aspect.

2.2 Testing Methods

The semi-dynamic leaching test is conducted as per ASTM C1308-08 [32]. Four replicate samples are tested with three different extraction leachants with pH = 2.0, 4.0, and 7.0, respectively. The ratio of the liquid volume to the solid superficial area is 9.5 (mL/cm²). The leachant is replenished at 2 h, 7 h, 1 d and then daily until 11 d. It is noted that the semi-dynamic leaching test is not conducted for the untreated soil as

a preliminary test shown that the untreated soil specimen disintegrated immediately after soaking in the leachant with pH of 7.0 (i.e., DDW).

The pH value of the leachate before each replenishment is measured using a pH meter HORIBA D-54. An aliquot of the leachate is filtered through a 0.45 µm filter and acidified to pH < 2.0 and the concentration of Pb is measured by inductively coupled plasma optical emission spectrometry (ICP-OES, PerkinElmer Optima 8000). Triplicate measurements of pH and Pb concentration are conducted for each sample and the averaged values are reported. The coefficient of variation (COV) values of the pH and Pb concentration for the triplicate measurements are < 3% indicating the good repeatability of the results. The dry density of each sample is calculated from the measured water content and density of two reduplicate stabilized soils before and after the semi-dynamic leaching test.

The cumulative mass of leached Pb is calculated by the following equation:

$$A_{i,Pb} = \sum c_i \times V_i \quad (1)$$

where $A_{i, Pb}$ = the cumulative mass of leached Pb after i th leaching (mg), c_i = the concentration of Pb after i th leaching (mg/L), and V_i = the volume of the leachate (L), which is 1.66 L in this test. The cumulative fraction of leached mass at time t (CFL) is calculated by:

$$CFL = \frac{A_{i,Pb}}{m} \quad (2)$$

where m = the total mass of Pb in the specimen (mg). The observed diffusion coefficient (D^{obs}) is calculated using the following equation:

$$D^{\text{obs}} = \frac{\pi}{4} \left(\frac{CFL}{\sqrt{t}} \cdot \frac{V}{S} \right)^2 \quad (3)$$

where V = volume of the specimen (cm^3), S = surface area of specimen (cm^2) and t = leaching time (s). Herein, D^{obs} is a retarded observed diffusion coefficient since it represents both diffusive and sportive properties of the soils [3, 33].

ANC test is performed according to the procedures developed by Stegemann and Côte [34]. Approximately 10 g soil is sampled from the hand broken sample cured under the standard condition for 28 days, crushed, sieved ($< 100 \mu\text{m}$), and mixed with 100 mL distilled water. A series of titration tests are conducted on the soil-distilled water mixture using 0.1 M nitric acid as the extraction liquid. An automatic titrating device (Auto Titrator ZDJ-4A) is used to fill extraction liquid until the leachate pH of mixture achieves the target value. Approximately 10 mL leachate after each titration test is collected and filtered through a $0.45 \mu\text{m}$ filter, and then the concentration of Pb measured by ICP-OES (PerkinElmer Optima 8000). The COV value of the added acid volume less than 6%. This test is performed in duplicate and the average results are reported.

The slope of the titration curve (i.e., acid added to the soil versus leachate pH is expressed as an index of β by Yong [35]:

$$\beta = -\frac{dC_A}{dpH} \quad (4)$$

where dC_A (cmol) = the increment of moles of acid added to the soil.

After the semi-dynamic leaching test, one specimen is used for MIP test conducted as per ASTM D4404 [36]. The MIP test is used to determine the pore size

distribution of the soil-binder mixture. Approximate 1 cm³ soil sample is collected from the specimen's surface to the same depth by carefully cutting with a stainless steel knife. Then the collected samples are frozen by the liquid nitrogen (boiling point is -195°C). The frozen samples are dried in a vacuum chamber under -80°C. The MIP tests are performed on dried samples using an Auto Proe IV 9510 mercury intrusion porosimeter. The pore diameter is calculated using the following capillary pressure equation according to ASTM D4404 [36]:

$$d = -\frac{4\tau \cos \alpha}{p} \quad (5)$$

where d (μm) = pore diameter; τ (N/m) = the surface tension; α (°) = contact angles of mercury with the solid; and p (N/m²) = applied pressure of mercury intrusion. In this study, the contact angle is set as 139° and surface tension value is set as 4.84×10⁻⁴ N/mm.

The XRD tests are performed on samples of GGBS-MgO paste and 18% GGBS-MgO stabilized kaolin spiked with 2% Pb that are cured under the standard condition for 28 days. Prior to the XRD analysis, 10 g sample is collected, air dried, ground, and sieved (< 0.075 mm), and frozen using liquid nitrogen (-195°C) to be dried by sublimation of the frozen water at -80°C. The XRD test is conducted on gold-coated samples on RigakuD/Max-2500 using a Cu-Kα source with a wavelength of 1.5405 Å. The instrument is operated at 40 kV and 20 mA. A step size of $2\theta = 0.02^\circ$ and a scanning speed of 5 s/step are used in the step scan mode. Samples are analyzed over a range of 2θ from 10° to 50°. The binder content, curing time, Pb concentration and number of replicate samples for the various tests are summarized in **Table 4**.

3 Results and Analyses

3.1 Dry Density

Table 5 shows the properties of the Pb-contaminated kaolin clay treated by different contents of GGBS-MgO before the semi-dynamic leaching test. The water content and porosity slightly decrease with increasing content of GGBS-MgO, whereas dry density values are practically the same regardless of GGBS-MgO content. **Tables 6** and **7** present the dry density and normalized dry density after leaching under different SAR pH conditions. It is shown that the change of both dry density and normalized dry density are insignificantly with GGBS-MgO content or SAR pH.

3.2 Cumulative Fraction Leached and Leachate pH

Fig. 1 shows the evolution of the cumulative Pb fraction (*CFL*) and leachate pH with time for samples with different GGBS-MgO contents under SAR pH = 2.0, 4.0, and 7.0 . It can be seen that the *CFL* gradually increase throughout the entire leaching time. When GGBS-MgO content increases from 12 to 18%, *CFL* decreases regardless of the SAR pH. The binder content only has a marginal influence on the *CFL* when it exceeds 15%. At the same binder content and time, the increments of *CFL* are much more significant when SAR pH decreases from 4.0 to 2.0 than those when SAR pH decreases from 7.0 to 4.0. The observation is consistent with those reported by [2] where PC is used as the binder for stabilizing Pb-contaminated kaolin soil. When the

SAR pH values are 4.0 or 7.0, the leachate pH curves are close to each other and both are approximately 10.5. When the SAR pH decreases from 4.0 to 2.0, a remarkable decrease of the leachate pH to ~2.5 is observed because the amount of the alkaline hydration products formed in the soil matrix is not sufficient to buffer the acid solution.

3.3 Observed diffusion coefficient

The cumulative Pb per cross-section area of the soil is plotted against $\log(t)$ in **Fig. 2** for different SAR pH values. The slopes of the regression lines are calculated in **Table 8**. According to USEPA (Method 1315), if the slope is close to 1 (slope > 0.65), surface dissolution will be the dominant leaching mechanism. If the slope is close to 0.5 ($0.35 < \text{slope} \leq 0.65$), diffusion is the leaching mechanism. If the slope is lower than 0.35, wash-off occurs (or depletion if it is found in the middle or at the end of the test). This study shows that for SAR pH = 2.0, the leaching mechanism is dissolution while it is diffusion for SAR pH = 4.0 and 7.0. The observed diffusion coefficients (D^{obs}) are in the range of 10^{-18} - 10^{-12} m²/s, which agree with the results from previous studies [37-41]. The comparisons of D^{obs} for Pb in GGBS-MgO, fly ash, quicklime-sulfate and PC stabilized soils [37-42] are shown in **Fig. 3**. Only a few D^{obs} values exist at leachant pH 2.0 due to the strong dissolution effect. When SAR pH is 4.0 or 7.0, D^{obs} decreases as the GGBS-MgO content increases from 12% to 18%. The main hydration products in the GGBS-MgO binder are calcium silicate hydrate (C-S-H), hydrotalcite-like phases (Ht) and brucite (Mg(OH)₂) if there is excess MgO

[20]. The formation of brucite and Ht causes a large solid volume expansion and fill pores in stabilized soils [20], leading to more compacted soil structure [2] and therefore lower D^{obs} with higher binder dosages. When the SAR pH decreases from 7.0 to 4.0, the more aggressive SAR attack on the hydration products leads to their gradual dissolution and consequently higher porosity (see “MIP test” section), resulting in slightly higher D^{obs} .

3.4 Acid neutralization capacity

Fig. 4(a) illustrates the titration curves for Pb-contaminated kaolin clay treated by GGBS-MgO. Before adding the acid, the soil with 12% GGBS-MgO binder displays a slightly lower leachate pH than that of the soil with 18% GGBS-MgO binder (i.e., soil pH = 9.9 versus 10.3). Higher GGBS-MgO binder content increases the initial soil pH and thereby could increase the resistance against acid attack. **Fig. 4(b)** shows the values of β computed by **Eq. (4)**. Values of β gradually decrease with leachate pH dropping from 10.0 to 5.0. The leachates with pH lower than 5.0 are not appropriate for evaluating the buffering capacity of the contaminated kaolin in this study since a certain amount of soil minerals might have dissolved in the ANC test [35]. Du et al. [14] stabilized Zn and Pb contaminated soils with a phosphate-based binder and observed the turning point of β occurs at the leachate pH of 5.0. In **Fig. 4(b)** the turning point of β is around 7.0 which is slightly higher than Du et al. [14] because the kaolin clay used in this study has less organic component and therefore less acid buffer capacity than natural clay used by Du et al. [14]. **Fig. 4(c)** shows the variation

of the leached Pb concentration ($\mu\text{g/kg}$ dry soil) with leachate pH obtained from the ANC test. When the leachate pH is in the range of 2.0 to 4.0, leached Pb concentration decreases noticeably with the increasing GGBS-MgO content or pH. In contrast, leached Pb concentration is lower than $0.015 \mu\text{g/kg}$ and the values are practically the same when the leachate pH ranges from 5.0 to 10.0 regardless of the GGBS-MgO content or leachate pH.

3.5 Pore size distribution

Fig. 5 presents the cumulative pore volumes for the Pb-contaminated kaolin clay treated by 12% and 18% GGBS-MgO under different pH conditions. Under the same SAR pH condition, the specimens stabilized with 12% GGBS-MgO have notably larger cumulative pore volume than the specimens stabilized with 18% GGBS-MgO. At the same GGBS-MgO content, the cumulative pore volume decreases as the SAR pH increases, which is more noticeable when SAR pH decreases from 4.0 to 2.0.

Fig. 6 shows the pore volumes for pore diameters in different ranges: $< 0.01 \mu\text{m}$ (intra-aggregate), 0.01 to $10 \mu\text{m}$ (inter-aggregate) and $> 10 \mu\text{m}$ (air pores) respectively. This classification of pore sizes is suggested by [Horpibulsuk et al. \[43\]](#) for the cement and fly ash-stabilized silty clay. The volume percentages of above classified pores are shown in **Table 9**. Regardless of the GGBS-MgO content, the proportions of air pores and intra-aggregate pores increase (more noticeable for air pores), whereas those of inter-aggregate pores decrease when the SAR pH decreases from 7.0 to 2.0. Increasing GGBS-MgO content is found to decrease the proportions of air pores while

increase those of inter- and intra-aggregate pores.

3.6 X-ray diffraction analysis

XRD tests are conducted on the GGBS-MgO paste and 18% GGBS-MgO stabilized kaolin spiked with 2% Pb to investigate the emerging reaction products in the GGBS-MgO paste and stabilized Pb-contaminated kaolin. The results are presented in **Fig 7**. For GGBS-MgO paste samples, the characteristic peaks of Ht at $2\theta \approx 11.5^\circ$ and 22.9° agree well with the findings of other researchers [20, 44]. In addition, C-S-H has been detected at $2\theta \approx 29.8^\circ$, 31.6° , 38.0° and 48.7° . The C-S-H has lower ratio of calcium and silicon, and therefore its peak is close to the calcite (CaCO_3) ($2\theta = 29.8^\circ$) as reported by previous studies [20, 44]. MgO is identified suggesting that it has not been fully consumed after 28 days of curing. For Pb-contaminated kaolin treated with 18% GGBS-MgO, the 2θ values of the C-S-H and Ht are found at $\sim 31.7^\circ$ and 11.3° respectively. The characteristic peak of quartz (SiO_2) has been detected at $2\theta \approx 33.6^\circ$. The 2θ of the kaolinite is detected at $2\theta \approx 12.6^\circ$, 19.8° and 34.8° . Additionally, a trace peak of lead carbonated hydroxide hydrate (hydrocerussite, $\text{Pb}_2(\text{CO}_3)_2(\text{OH})_2$) has been detected at $2\theta = 34.2^\circ$, which agrees well with Jin and Al-Tabbaa (2014a) [20] that the main immobilization mechanism for Pb by GGBS-MgO binder is the formation of hydrocerussite.

4 Discussion

This study demonstrates that the SAR pH and GGBS-MgO content affect

considerably the leachability and D^{obs} of Pb and microstructural properties of the GGBS-MgO stabilized Pb-contaminated kaolin clay. The mechanisms controlling the variation of these features are summarized as follows:

(1) As the SAR pH decreases from 7.0 to 2.0, the hydration products (C-S-H and Ht) and kaolin might have been gradually dissolved although it is not explored in this study. The treated Pb-contaminated soils subjected to the acidic conditions (pH 4.0 to 2.0), therefore, possess looser structures as compared to those subjected to the neutral condition (pH 7.0) (**Fig. 6**). As the GGBS-MgO content increases from 12% to 18%, higher amounts of hydration products have formed in the soil matrix [15] which in turn results in lower leached Pb concentration observed in the ANC test (**Fig. 4(c)**) and reduces the cumulative volume of pores and the proportions of air pores obtained from the MIP analyses (**Fig. 5**). More compact structure leads to lower D^{obs} and CFL values of Pb in the stabilized soils [2].

(2) Regarding the relative variations of the pore volumes in different sizes (**Table 9**), it is proposed that: (a) acid attack results in disintegration of the soil-binder aggregates because kaolin particles and hydration products filling the intra-aggregate pores dissolve, and therefore the intra-aggregate pores volume is elevated; (b) the aggregates themselves, which are formed by kaolin-cement clusters, gradually and partially dissolve due to the acid attack, resulting in the transformation of inter-aggregate pores to air pores. Hence, the inter-aggregate pores volume decreases with decreasing SAR pH, whereas the air pores volume increases with decreasing SAR pH.

(3) Different SAR pH conditions and GGBS-MgO contents affect the soil acid buffering capacity (**Fig. 4**). The β values decrease with decreasing pH because the more hydration products are dissolved with more free hydrogen ions (H^+) in the extraction liquid. As the GGBS-MgO content increases from 12% to 18%, a higher concentration of free hydroxyl ions (OH^-) are produced in the soil pores due to cement hydration, resulting in higher β values.

(4) Both soil structure and acid buffering capacity affect CFL and D^{obs} of Pb. The above analyses show that soils exhibit loose structures and low β values when SAR pH or GGBS-MgO content reduces, which in turn results in elevated CFL values (**Fig. 1**). **Eq. (3)** shows that D^{obs} obtained from the semi-dynamic test has a square relationship with CFL . Consequently, D^{obs} values increase with the decreasing SAR pH or GGBS-MgO content (**Table 8**). At a strong acidic condition (pH 2.0), leaching of Pb is controlled by the mineral dissolution process and therefore D^{obs} is not available.

It should be noted that the stabilized soils are cured for 28 d in this study, while it is demonstrated that the mechanical properties/microstructure of GGBS-MgO improve significantly in the long term (> 90 days) [15, 45]. Therefore longer curing time is warranted to fairly evaluate the performance of GGBS-MgO stabilized contaminated soils. Moreover, the tested soil samples are prepared under well-controlled laboratory conditions, so it is suggested that field contaminated soils could be adopted in future studies.

5 Conclusions

This study investigates the effect of acid rain with different pH values on the leaching properties of GGBS-MgO stabilized Pb-contaminated kaolin clay via a series of semi-dynamic leaching tests. The effects of the pH of simulated acid rain (SAR) and GGBS-MgO content on the cumulative fraction leached (CFL), observed diffusion coefficient of Pb and pore size distribution profiles of the soils are investigated. Based on the results obtained from this study, the following conclusions can be drawn:

(1) The changes of dry density and normalized dry density of Pb-contaminated kaolin clay with GGBS-MgO content or acid rain pH are insignificant within the test conditions in this study.

(2) The *CFL* of Pb is notably affected by the simulated acid rain pH and GGBS-MgO content. *CFL* decreases with increasing GGBS-MgO content while increases with decreasing pH, and its increment is more notable at pH 2.0. At pH 2.0, mineral dissolution is found to be the dominant process that controls the leaching of Pb.

(3) The dominant leaching mechanism of Pb is diffusion at pH 4.0 and 7.0. D^{obs} values of Pb decrease with increasing GGBS-MgO content. The results obtained from this study and those reported in the literature demonstrate that D^{obs} values of Pb increases with decreased pH of leachant tested in the leaching tests.

(4) The Pb-contaminated soils with higher GGBS-MgO content display flatter acid neutralization capacity titration curves and higher acid buffer capacity index.

When the leachate pH ranges from 2.0 to 4.0, the leached Pb concentration obtained from the ANC test decreases noticeably with increasing GGBS-MgO content or leachate pH, while it changes insignificantly regardless of GGBS-MgO content or leachate pH when pH is in the range of 5.0 to 10.0.

(5) MIP results show that the cumulative pore volume decreases as the simulated acid rain pH or GGBS-MgO content increases. The volume percentages of air pores and intra-aggregate pores increase while that of inter-aggregate pores decreases with decreased SAR pH. Increasing GGBS-MgO content decreases the volume percentages of air pores while increases those of both inter- and intra-aggregate pores.

(6) Lead is primarily precipitated as hydrocerussite ($\text{Pb}_2(\text{CO}_3)_2(\text{OH})_2$) in the soil matrix after GGBS-MgO treatment. The acid buffer capacity and pore structure obtained from the ANC test and MIP analyses are essential for interpreting effects of leachant pH and GGBS-MgO content on the leachability and diffusive properties of Pb in the soils tested.

Acknowledgments

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Table Captions

Table 1. Properties of the kaolin soil used in this study

Table 2. Main physico-chemical properties of GGBS and MgO

Table 3. Chemical compositions of the kaolin soil, GGBS and MgO used in this study
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Table 5. Properties of the stabilized soil before the semi-dynamic leaching tests

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Table 9. Distribution of the pore volume percentage of the Pb contaminated kaolin
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612

Table 1. Properties of the kaolin soil used in this study

Index	Value
pH	8.77
Specific gravity, G_s	2.68
Plastic limit, w_P (%)	14.6
Liquid limit, w_L (%)	29.4
Grain size distribution (%)	
Clay (< 0.002 mm)	21.5
Silt (0.002 to 0.075 mm)	58
Sand (0.075 to 2 mm)	20.5

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Table 2. Main physico-chemical properties of GGBS and MgO

Property	Value	
	GGBS	MgO
Alkalinity ^a	1.689	-
Reactivity (s)	-	102
Specific surface area (m ² /g)	0.2932	28.023
pH (liquid to solid ratio = 1:1)	10.96	10.53

615 ^aThe alkalinity of the GGBS is defined as the ratio of contents of CaO, MgO, and
616 Al₂O₃ to that of SiO₂
617

Table 3. Chemical compositions of the kaolin soil, GGBS and MgO used in this

study measure by XRF

Chemical composition (<i>wt%</i>)	Kaolin	GGBS	MgO
CaO	0.36	33.08	0.84
Al ₂ O ₃	39.3	17.9	0.38
MgO	0.06	6.02	96.5
K ₂ O	0.21	0.64	0.01
SiO ₂	52.1	34.3	1.09
Fe ₂ O ₃	3.38	1.02	0.19
SO ₃	0.06	1.64	0.26
MnO	0.11	0.28	0.02
TiO ₂	1.12	0.92	0.01
Loss on ignition (at 950°C)	3.3	4.2	0.7

Table 4. Binder content, curing times, Pb concentration and number of replicated samples for various tests used in this study

Test/ Analysis	Binder content, %	Curing time, days	Pb concentration, %	Number of replicate samples
Dry density	12, 15, 18	28	2	2
Semi-dynamic test	12, 15, 18	28	2	4
ANC ^a	12, 18	28	2	1
MIP ^b	12, 18	28	2	1
XRD ^c	18	28	0, 2	1

^aANC= Acid neutralization capacity;

^bMIP = Mercury intrusion porosimetry;

^cXRD = X-ray diffraction

Table 5. Properties of the stabilized soils before the semi-dynamic leaching tests

	^a Specific gravity, G_s	^b Water content, %	Dry density, ρ_d (g/cm ³)	Porosity, n	Saturation degree, S
GM12Pb2	2.61	41.0	1.26	0.519	0.992
GM15Pb2	2.59	39.7	1.26	0.514	0.973
GM18Pb2	2.55	38.2	1.28	0.498	0.982

^a ASTM D5550([ASTM 2014](#))

^b ASTM D2216([ASTM 2010](#))

Table 6. Dry density (g/cm^3) of samples calculated from the measured density and water content immediately after the semi-dynamic leaching tests

Sample / pH	pH = 2.0	pH = 4.0	pH = 7.0
GM12Pb2	1.20	1.22	1.23
GM15Pb2	1.25	1.25	1.26
GM18Pb2	1.25	1.25	1.26

Table 7. Normalized dry density (g/cm^3) of samples calculated from the measured density and water content immediately after the semi-dynamic leaching tests

Sample / pH	pH = 2.0	pH = 4.0	pH = 7.0
GM12Pb2	0.96	0.97	0.98
GM15Pb2	0.99	0.99	1.00
GM18Pb2	0.98	0.98	0.99

Table 8. Calculated D^{obs} values and leaching mechanisms of Pb for the soils tested between 2 h and 11 d

	pH	Slope	R^2	Mechanism	D^{obs} (m ² /s)
GM12Pb2	2.0	0.71	0.992	dissolution	-
	4.0	0.51	0.96	diffusion	1.69×10^{-16}
	7.0	0.44	0.997	diffusion	7.67×10^{-18}
GM15Pb2	2.0	0.8	0.989	dissolution	-
	4.0	0.52	0.972	diffusion	4.90×10^{-17}
	7.0	0.58	0.962	diffusion	5.00×10^{-18}
GM18Pb2	2.0	0.98	0.982	dissolution	-
	4.0	0.6	0.98	diffusion	5.07×10^{-17}
	7.0	0.6	0.948	diffusion	3.77×10^{-18}

Table 9. Distribution of the pore volume percentage of the Pb contaminated

kaolin treated by GGBS-MgO after semi-dynamic test

	GM12Pb2			GM18Pb2		
SAR pH	pH = 2.0	pH = 4.0	pH = 7.0	pH = 2.0	pH = 4.0	pH = 7.0
intra-aggregate	1.86	1.80	1.77	2.74	2.00	1.76
pores (%)						
inter-aggregate	90.76	91.18	91.70	91.00	92.15	93.86
pores (%)						
air pores (%)	7.38	7.00	6.53	6.26	5.85	4.38

Figure Captions

Figure. 1. *CFL* for Pb and leachate pH for soils with GGBS-MgO contents of (a) 12%; (b) 15%; and (c) 18%

Figure. 2. The plots of cumulative Pb against $\log(t)$ from the semi-dynamic tests for specimens with GGBS-MgO content of (a) 12%; (b) 15%; and (c) 18%

Figure. 3. Variation of D^{obs} for lead with leachant pH obtained from this study and previously published studies

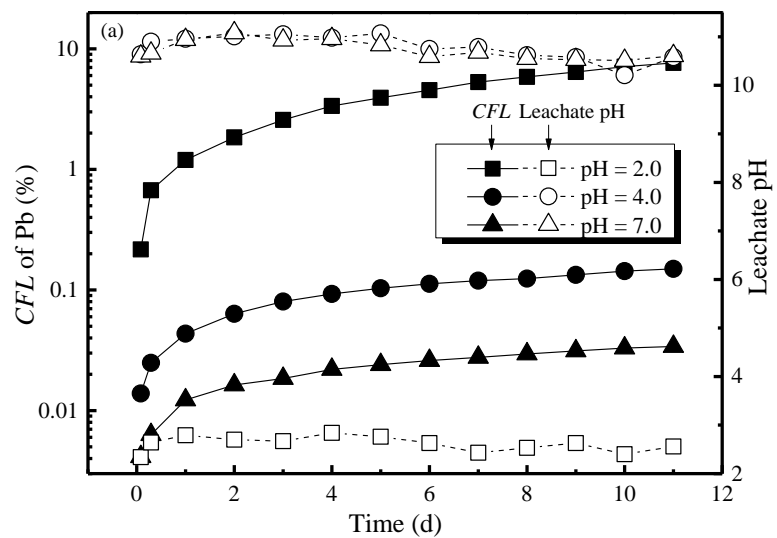
Figure. 4. (a) Acid titration curves; (b) buffer capacity; and (c) leached Pb concentration of the lead contaminated kaolin clay treated by GGBS-MgO

Figure. 5. Cumulative pore volume of the Pb contaminated kaolin treated by GGBS-MgO after semi-dynamic leaching test

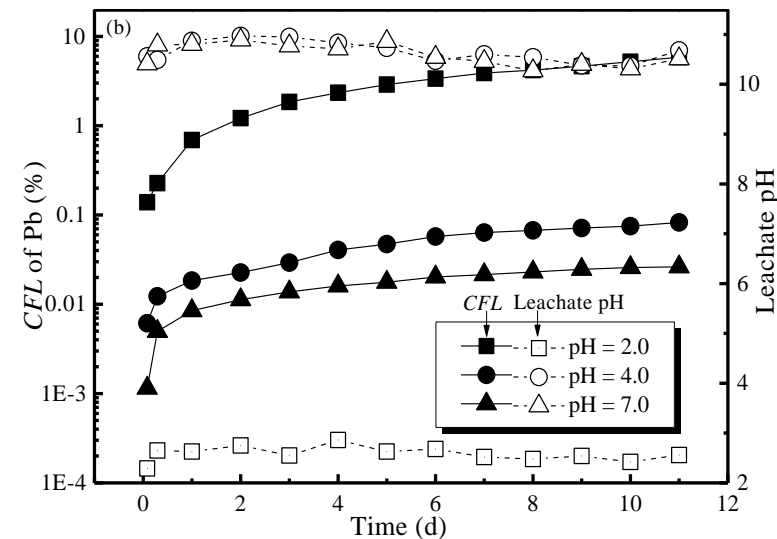
Figure. 6. Pore volume percentage of the Pb contaminated kaolin treated by GGBS-MgO after semi-dynamic test

Figure. 7. X-ray diffractograms of the GGBS-MgO paste and GGBS-MgO treated Pb-contaminated kaolin cured at the normal condition for 28 days

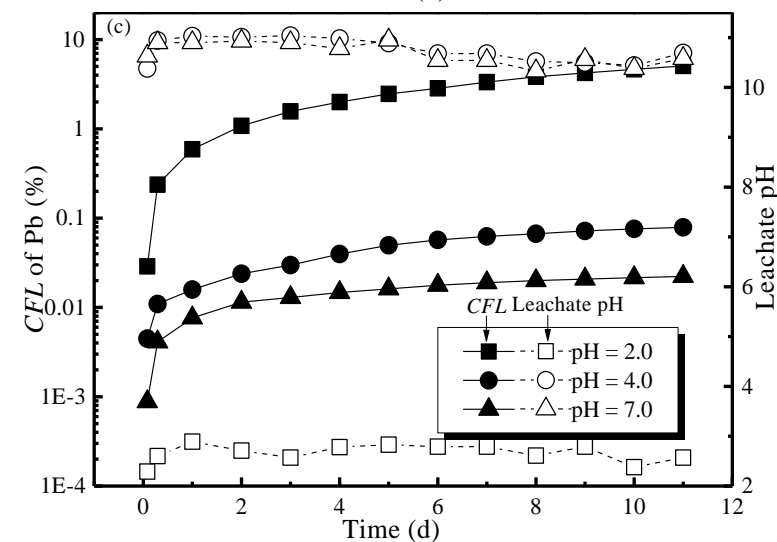
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Fig. 1 CFL for Pb and leachate pH for soils with GGBS-MgO contents of (a)

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12%; (b) 15%; and (c) 18%.

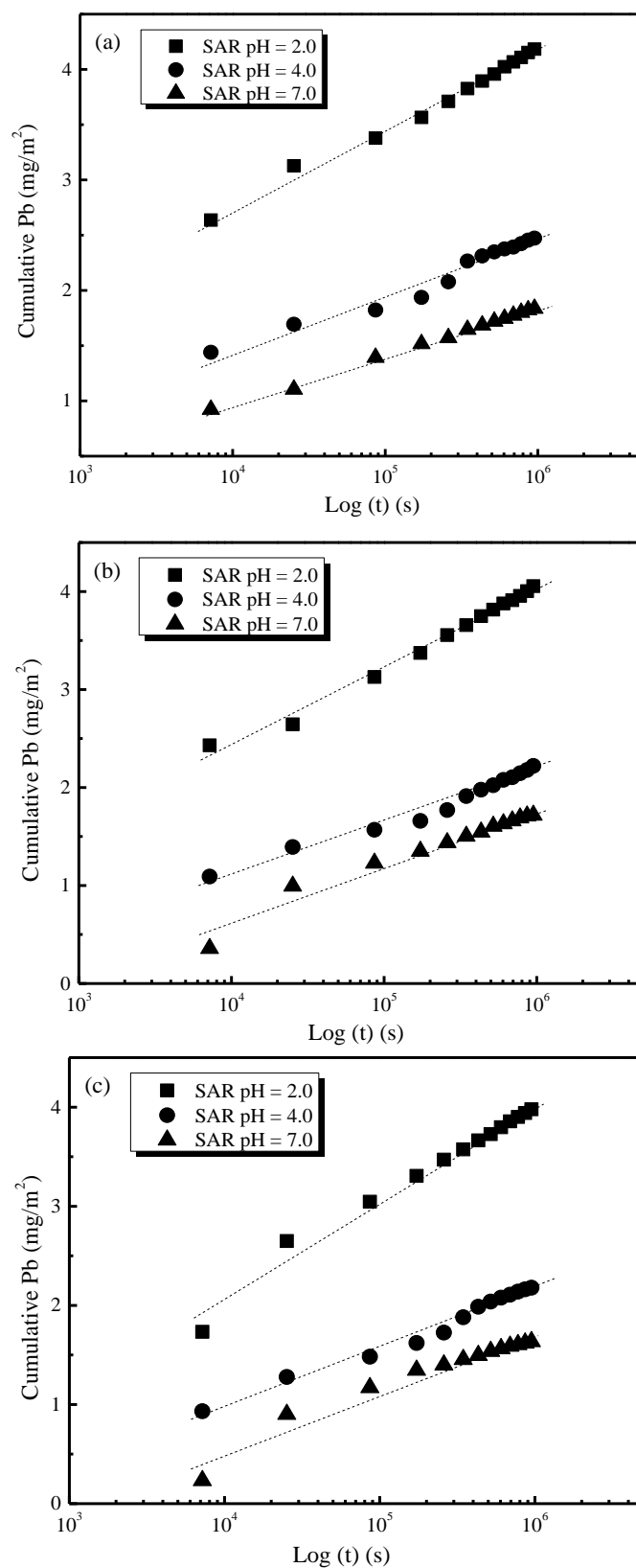


Fig. 2 The plots of cumulative Pb against log (t) from the semi-dynamic tests for specimens with GGBS-MgO content of (a) 12%; (b) 15%; and (c) 18%.

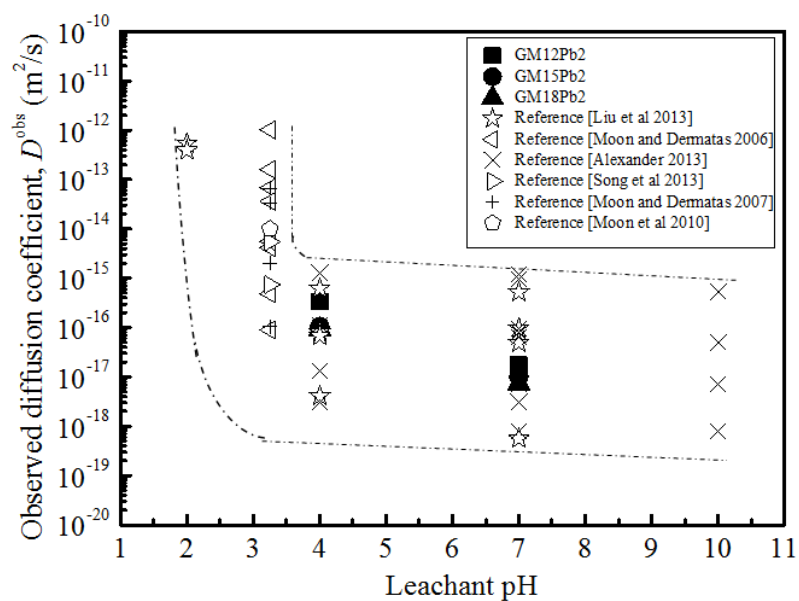


Fig. 3 Variation of D^{obs} for lead with leachant pH obtained from this study and previously published studies

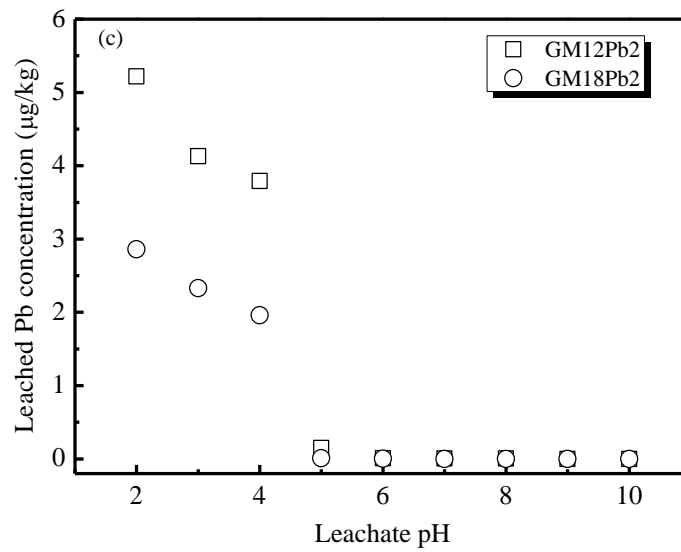
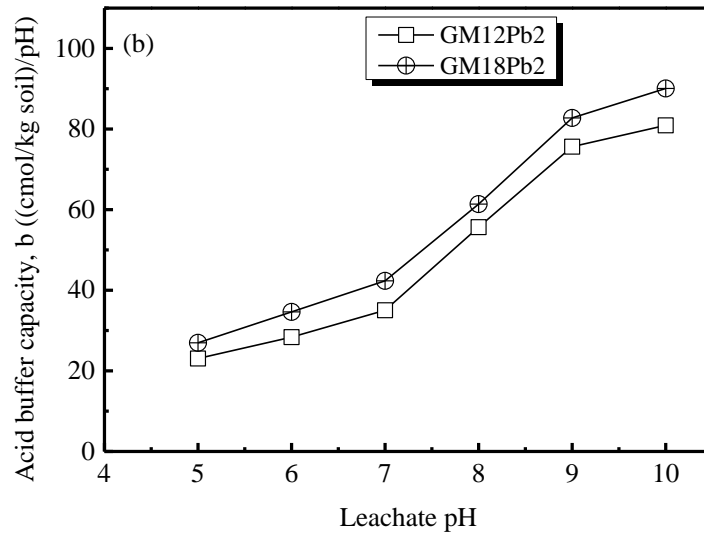
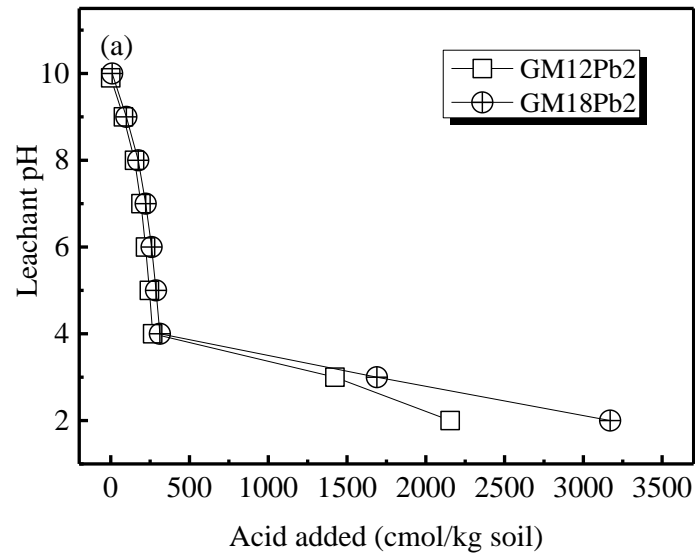


Fig. 4 (a) Acid titration curves; (b) buffer capacity; and (c) leached Pb concentration of the lead contaminated kaolin clay treated by GGBS-MgO

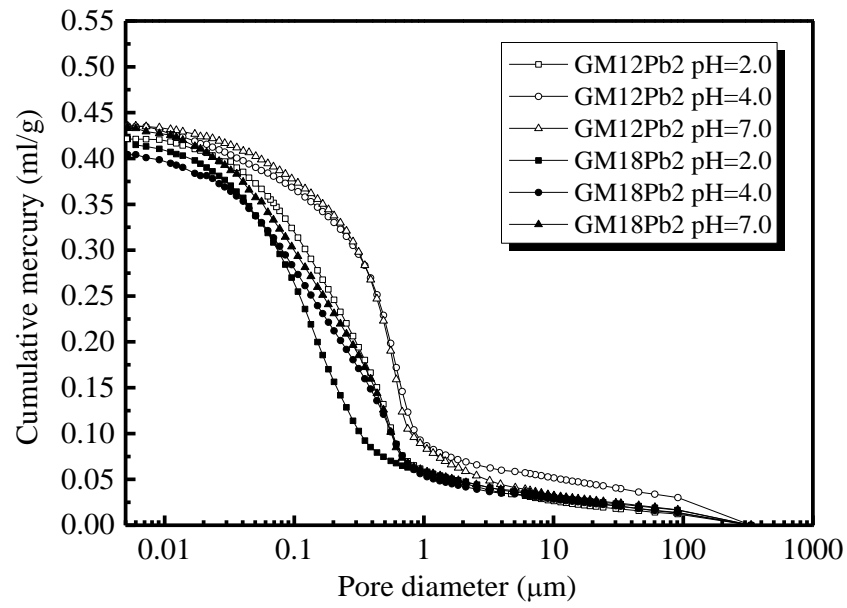


Fig. 5 Cumulative pore volume of the Pb contaminated kaolin treated by GGBS-MgO after semi-dynamic leaching test

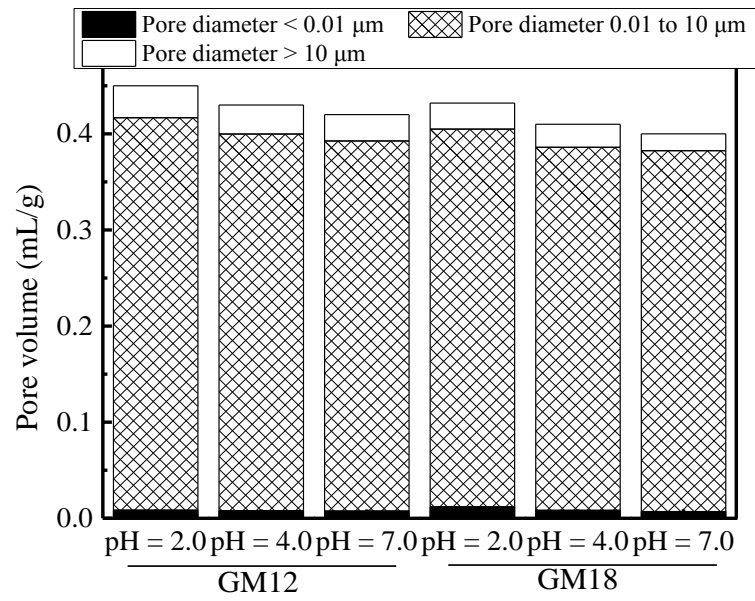


Fig. 6 Pore volume percentage of the Pb contaminated kaolin treated by GGBS-MgO after semi-dynamic test

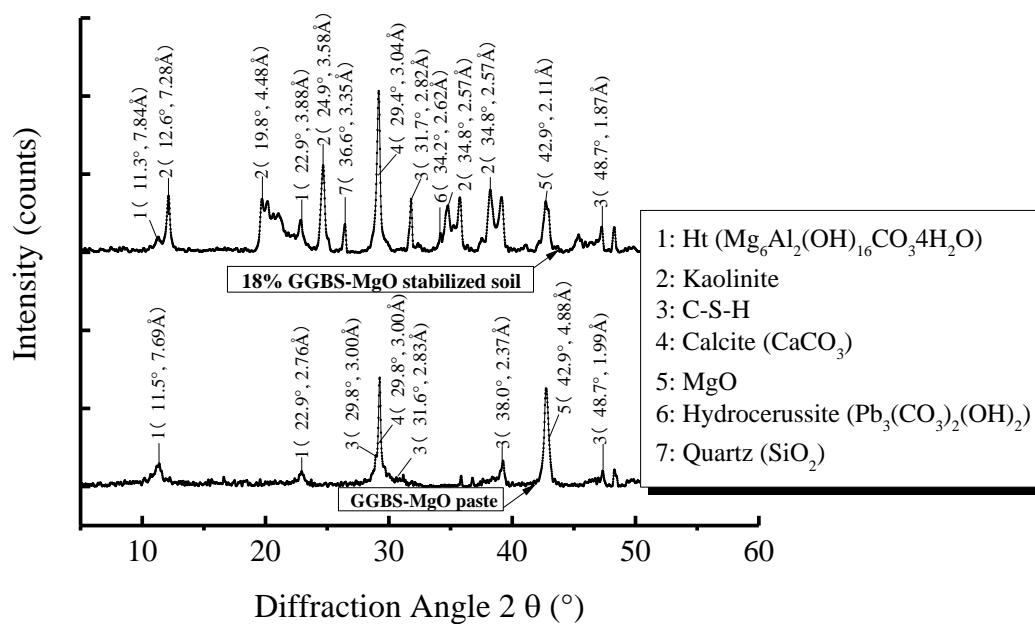


Fig. 7 X-ray diffractograms of the GGBS-MgO paste and 18% GGBS-MgO treated Pb-contaminated kaolin cured under standard condition for 28 days