

1     **REFINEMENTS TO THE EFDC MODEL FOR PREDICTING THE HYDRO-**  
2     **ENVIRONMENTAL IMPACTS OF A BARRAGE ACROSS THE SEVERN**  
3     **ESTUARY**

4                     JUNTAO ZHOU (1), ROGER A. FALCONER (1), BINLIANG LIN (1, 2)

5     (1): *Hydro-environmental Research Centre, School of Engineering, Cardiff University, Cardiff CF24*  
6                     *3AA, UK*

7     (2): *State Key Laboratory of Hydrosience and Engineering, Tsinghua University, Beijing 100084,*  
8                     *China*

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10    **ABSTRACT**

11       This paper presents an investigation of the impacts of a Severn Barrage on the hydro-environment of  
12    the Bristol Channel and Severn Estuary using the Environmental Fluid Dynamics Code (EFDC) model  
13    with a recently developed Barrage module (EFDC\_B). Details are given of a barrage module being  
14    implemented into the EFDC model to represent the various hydraulic structures, such as turbines and  
15    sluice gates, as deployed along the barrage line. Several cases, both with and without the barrage, have  
16    been simulated to investigate the potential changes on the peak water levels, minimum water depths  
17    and peak tidal currents arising from a barrage. The impacts of a barrage on the salinity concentration  
18    distribution have also been simulated in both 2D and 3D modes. The predicted results showed that the  
19    maximum water levels could be significantly reduced, especially downstream of the barrage and for  
20    much of the region in the Severn Estuary and that the minimum water depths would be changed so  
21    much that there would be 80.5 km<sup>2</sup> loss of intertidal habitats due to the sitting of a barrage across the  
22    estuary. Likewise, the peak tidal currents would be considerably reduced, and by as much as a half in  
23    the middle of the main channel. The predicted salinity concentrations results indicated that at high  
24    water, the salinity concentrations would be reduced by 1-2 ppt downstream and upstream of the barrage  
25    and salinity concentrations in the region near Beachley would be reduced by up to 5 ppt, and that at low  
26    water, salinity concentrations would be reduced by 0.5 – 1 ppt in the middle of the Bristol Channel and  
27    by typically 0.5 ppt and 1 ppt downstream and upstream of the barrage, respectively. The predicted  
28    results also indicated that salinity concentrations downstream and upstream of the barrage would be  
29    under a stable state with slight oscillations all the time due to the effects of the barrage. A comparison  
30    between the salinity concentration distributions predicted by the 2D and 3D models indicated that the

1 two models produced similar salinity distributions, especially in the Severn Estuary and in the region  
2 between the middle of the Bristol Channel and the seaward open boundary.

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4 | KEY WORDS: Tidal Barrage; Numerical modelling; EFDC\_B model; Hydrodynamics; Salinity; Tidal  
5 | Power; 3D effects

## 6 7 **1. INTRODUCTION**

8 Over the past few decades, the energy demand has been continuously increasing as a result of  
9 developing economies, particularly in China and India, and population growth. On the other hand, due  
10 to the over-dependence on traditional energy sources (such as fossil fuels and natural gas), global  
11 reserves are quickly depleting and hence the price of traditional energy has increased significantly.  
12 Compared with traditional energy resources, renewable energy resources have many advantages, such  
13 that increasing efforts are now being made to increase the provision of such resources. Among the  
14 range of renewable resources available, tidal power has great potential for future electricity generation,  
15 since tides are predictable compared to wind energy, solar power etc. (Falconer et al. [1]).

16 With one of the highest tidal ranges in the world occurring in the Bristol Channel and Severn Estuary,  
17 located in the southwest of the UK (see Fig. 1), this basin is regarded as an ideal location for potential  
18 tidal energy extraction. Therefore, it is significant to predict numerically the effects of a tidal barrage  
19 on the hydro-environment of the Bristol Channel and Severn Estuary. Recently, several numerical  
20 models have been used to study the tidal hydrodynamic processes and tidal barrage impacts on the  
21 aquatic environment in the Severn Estuary. Owen [2] and Stephens [3] used 3D regular grid numerical  
22 models to predict tidal levels and currents in the Bristol Channel, with the horizontal grid size being  
23 over 1 km. According to the observed current data from 1975 to 1977, Uncles and Jordan [4] derived  
24 co-phase and co-amplitude lines for the M2 tide and obtained estimates of the associated tidal energy  
25 fluxes through selected cross-sections. Evans et al. [5] investigated the effect of a barrage on the water  
26 quality parameters in the estuary using a depth-averaged 2D hydrodynamic model. In this study four  
27 types of nested grids of square cells were deployed to represent the topography, with the finest grid size  
28 being 0.167 km covering the proposed barrage site. Amin and Flather [6] dynamically linked a 2D  
29 numerical model of the Bristol Channel to a 1D model of the River Severn and compared the numerical  
30 model predicted water levels with the corresponding values predicted based on a harmonic analysis.

1 More recently, Xia *et al.* [7, 8, 9] deployed a 2D finite volume numerical model to investigate the  
2 impact of different tidal renewable energy schemes on the hydrodynamic processes. Due to the  
3 advantages of an unstructured triangular mesh, as used in this model, the model accurately fitted the  
4 complex and irregular land boundary and especially around the barrage structure, with accurate  
5 simulations and predictions of the hydrodynamic processes in the region of the structure being  
6 achieved. However, one of the disadvantages of an unstructured grid is that it not only needs to  
7 generate a grid and to process and store the “connectivity” information, but also that every node has a  
8 different scattering behaviour, which incurs both a memory and speed penalty, especially if a 3D model  
9 is required. Ahmadian *et al.* [10] used a regular grid model, namely the DIVAST model, to investigate  
10 the hydrodynamic and corresponding suspended sediments and bacterial fluxes in the Bristol Channel  
11 and Severn Estuary. Good agreement between the predicted results and field data were obtained based  
12 on a fine grid distributed throughout the whole computational domain. Undoubtedly improvements in  
13 the accuracy were obtained at the expenses of computational efficiency in these numerical model  
14 simulations. Furthermore, as the Severn Estuary becomes very narrow and meanders upstream of the  
15 Severn Bridge, then computational efficiency is very low if the same 2D grid resolution is used along  
16 the estuary. To overcome this problem, a linked 1D-2D model was used, with the linked model being  
17 allowed to overlap for a distance to ensure numerical stability between the two models. The linked 1D–  
18 2D model may yield an inconsistent problem of accuracy between the 1D and 2D models, which  
19 further affects the accuracy of an integrated model solution.

20 Hence, a curvilinear orthogonal grid model, namely EFDC, has been used and presented in this study.  
21 A curvilinear orthogonal mesh can accurately represent the complex coastline and estuarine boundaries,  
22 without the need for any model coupling and has the advantage of computational efficiency over  
23 unstructured grid models.

24

## 25 **2. DESCRIPTION OF EFDC MODEL**

26 The EFDC model [11] is a general purpose modelling package for simulating three-dimensional  
27 flow, transport, and biogeochemical processes in surface water systems, including: rivers, lakes,  
28 estuaries, reservoirs, wetlands and near-shore to shelf-scale coastal regions. The EFDC model can  
29 predict the water level and velocity, salinity and temperature distributions and also simulates the  
30 processes of cohesive and non-cohesive sediment transport, near field and far field discharge dilution

1 from multiple sources, eutrophication, the transport and fate of toxic contaminants in the water and  
2 sediment phases, and the transport and fate of various life stages of finfish and shellfish. EFDC also  
3 allows for drying and wetting in shallow areas using a mass conservative scheme. A number of  
4 alternatives are in place in the model to simulate general discharge control structures, such as weirs,  
5 spillways and culverts. The model uses either stretched or sigma vertical coordinates and Cartesian or  
6 curvilinear orthogonal horizontal coordinates. Dynamically coupled transport equations for turbulent  
7 kinetic energy, turbulent length scale, salinity and temperature are also solved. The two turbulence  
8 parameter transport equations based on the Mellor-Yamada level 2.5 turbulence closure schemes are  
9 used [12]. A second-order accurate spatial finite difference formulation on a staggered (or C) grid is  
10 used to solve the momentum equations, while a second-order accurate three-time level finite difference  
11 scheme is used, with an internal-external mode splitting procedure adopted to separate the internal  
12 shear, or baroclinic mode, from the external free surface gravity wave, or barotropic mode. A  
13 numerical solution based on a second-order accurate scheme in both space and time, and a mass  
14 conservative fractional step algorithm is used to solve the transport equations for salinity, temperature  
15 and other constituents. More details of the governing equations for the original EFDC hydrodynamic  
16 model equations and the numerical schemes can be found in Hamrick, [11]. The EFDC model is a  
17 public domain software, and has been extensively tested and applied in more than 100 modelling  
18 studies worldwide. The model has been successfully applied to reservoirs and lakes [13], bays and  
19 estuaries [14], [15], [16] and [17] to study the hydrodynamic and water quality processes [18], tidal  
20 intrusion and sediment transport and its impact on the fate of faecal bacteria [19].

21

### 22 **3. BARRAGE MODULE DEVELOPED FOR EFDC MODEL**

23 In this study the main aim has been to assess numerically the impact of the Severn Barrage on the  
24 hydro-environment in the Severn Estuary and Bristol Channel. Hence a barrage module has been  
25 developed for inclusion within the EFDC model to represent the operation of a barrage. The numerical  
26 model EFDC with a barrage module can be abbreviated to EFDC\_B model. There are two main types  
27 of devices frequently included in barrages for connecting the conditions upstream and downstream of  
28 the structure, i.e. sluice gates and turbines. The numerical implementation of a sluice gate is similar to  
29 the procedure that occurs in reality. When the sluice gates need to be closed, then the current velocities  
30 through the computational cells representing the sluice gates are set to gradually decrease to zero.

1 Likewise when the sluice gates need opening, the velocities are set to gradually increase to the peak  
2 velocity. Hydraulic structures have been adopted to represent the turbines. In EFDC each hydraulic  
3 structure has an upstream end and a downstream end and water can only flow from upstream to  
4 downstream of the hydraulic structure, thus two hydraulic structures are required in order to simulate  
5 water passing through the barrage. The upstream and downstream conditions of the hydraulic  
6 structures have been linked dynamically using a  $Q$ - $H$  relationship. The discharge  $Q$  through a turbine  
7 was obtained by linking  $Q$  with the difference between the upstream and downstream water head as  
8 given by:

$$10 \quad Q = C_d A \sqrt{2gH} \quad (1)$$

11  
12 where  $C_d$  is the discharge coefficient of a turbine;  $A$  is the area of the opening and  $H$  is the head  
13 difference across the turbine. For the Severn Barrage Severn Tidal Power Group study [20] where the  
14 turbine diameter was assumed to be 9 m, and the mean wet flow-through area of each turbine was 63.6  
15 m<sup>2</sup>. The formula for computing the generation power of  $P$  can be expressed by

$$17 \quad P = \rho g Q H \eta \quad (2)$$

18  
19 where  $\rho$  is the specific density of sea water (kg/m<sup>3</sup>); and  $Q$  is the discharge across the turbines (m<sup>3</sup>/s);  
20  $\eta$  is the efficiency coefficient of the turbines and  $g$  is gravitational acceleration. In the current study  
21 the discharge coefficient  $C_d$  was assumed to be 1 for turbines and the efficiency coefficient  $\eta$  was  
22 assumed to be 1 [8] and [21].

23 The Severn Barrage scheme proposed by the Severn Tidal Power Group [20] would stretch nearly 16  
24 km from Lavernock Point, southwest of Cardiff, to near Brean Down, near Weston-Super-Mare. This  
25 structure included 216 bulb turbines and 166 sluices, with a detailed layout of this barrage  
26 configuration being given in Fig 2. The energy operating scheme for this original Severn Barrage  
27 layout was designed as a one-way generation process [20], i.e. only ebb generation was considered  
28 suitable for this Barrage. Ebb generation usually has four stages, namely: filling, holding (i), generating  
29 and holding (ii), respectively. When the barrage is at the filling stage, the sea water flows through the

1 sluices and turbines, raising the water level upstream of the barrage until high water level is reached  
2 and then the sluice gates and turbines are closed. For the holding stage (i): both the turbines and sluice  
3 gates are kept closed until there is sufficient head difference (i.e. the starting head) created between the  
4 downstream and upstream sides of the barrage due to the sea level fall; the turbines are then switched  
5 on to generate power until the head difference is lower than the minimum water head for turbine  
6 operation. At the last stage (i.e. the holding stage (ii)), both the turbines and sluice gates are closed  
7 again until the downstream sea level is greater than the basin level, and another filling stage then  
8 commences.

9 In order to obtain the same computational accuracy between the barrage and no-barrage cases, the  
10 same mesh has been used in the cases with and without a barrage except in the region of the barrage.  
11 Furthermore, for accurate simulations of the barrage operating scheme and the complex flow patterns  
12 around the barrage, a fine grid was deployed in the region surrounding the barrage with the total cell  
13 number being 53,914 for the case with the barrage and with the representative grid size around the  
14 barrage being 70m x 100 m. Fig. 3 shows the distribution of the fine grid and the corresponding device  
15 locations along the barrage line. In this study, the hydraulic structures such as the ship locks, sub-  
16 stations and embankments along the barrage line were all treated as wall boundaries. Due to the instant  
17 opening and closing of the turbines and the sluice gates, some numerical oscillations in water  
18 elevations can be observed when the barrage is in operation. To remove these numerical oscillations, a  
19 ramp function was introduced to change the flow volume through the turbines and sluice gates during  
20 the opening and closing of turbines and sluice gates, as would occur in reality. Assuming that it takes a  
21 time period of  $T$ , required to open the turbine valves (or sluice gates), the ramp function is

22  $f = \sin\left(\frac{\pi t}{2T}\right), 0 < t \leq T$ , where  $T = 10$  minutes in this study (based on typical opening times cited

23 in commercial literature), and  $t$  is time. When the turbine valves (or sluice gates) start to open,  $t=0$  and  
24  $f=0$ ; when they are fully opened,  $t=T$  and  $f=1$ . Likewise, when these hydraulic structures need closing,

25 the ramp function is changed into  $f = \cos\left(\frac{\pi t}{2T}\right), 0 < t \leq T$ . Based on different operation procedures of

26 the barrage and the head differences downstream and upstream of the barrage, these can be used to  
27 decide what time to open the turbine valves and sluice gates or to close them, and then the flow  
28 volumes through the turbines and sluice gates are gradually increased or decreased by multiplying the  
29 discharge by the ramp function.

1

## 2 **4. MODEL DETAILS AND VALIDATION**

3 The bathymetry of the computational domain is shown in Fig. 4, where the elevation data are  
4 provided relative to the Ordnance Datum (OD). The total model area is about 5700 km<sup>2</sup> which covers  
5 the whole of the Bristol Channel and the Severn Estuary. The western boundary, i.e. the open  
6 boundary, was set between Hartland point in England and Stackpole Head in Wales, with the time  
7 varying water level being specified along this boundary. The landward boundary was set at the River  
8 Severn tidal limit, located close to Gloucester, to account for the possible impact of the Severn Barrage  
9 on the tidal levels of the River Severn. The corresponding water level at the open boundary was  
10 specified using the predicted elevation data from POLPRED (Xia et al., 2010a). The simulation  
11 duration was 300 hours, starting from 5:30pm on 20<sup>th</sup> July, 2001 to 5:30am 2<sup>nd</sup> August, 2001  
12 (excluding the salinity flux studies). The total CPU time for this case was 2.64 hours using a PC with  
13 Intel (R) Core (TM) 8 CPU @ 2.4 GHz (only one CPU was used) and 24G RAM. Only one river was  
14 considered in the fresh water inflow simulations in this study, namely the River Severn, as this was the  
15 most substantial river in terms of water flow into the basin from England and Wales. A constant base  
16 flow rate of 100 m<sup>3</sup>/s was used in this study, for the River Severn inflow boundary at Gloucester. It also  
17 can be seen from Fig. 4 that the gradient of seabed elevation is relatively steep, with the average bed  
18 elevation decreasing from about -60 m near the open seaward boundary to about -10 m near  
19 Avonmouth. Furthermore, the surface width also decreases greatly throughout the whole computational  
20 domain, from about 72 km at the open seaward boundary to about 130 m at the landward boundary at  
21 Gloucester. Because of the bell-mouthed shape of the estuary, the irregular land boundary and the high  
22 tidal range, it is always a challenge for any numerical model to obtain a stable and accurate simulation  
23 for this estuary. Comparisons between the field data and numerical results have been made in this  
24 section to illustrate that good agreement has been achieved using the EFDC model. Based on a  
25 sensitivity analysis of this model for different time steps, a constant time step of 5 s was chosen for the  
26 simulations.

27

### 28 4.1 Validation of tidal level

29 The model predicted water levels were first validated against the field data. Three typical sites were  
30 chosen for the comparisons, i.e. the Mumbles, Newport and the Hinkley Point, which are located

1 upstream and downstream of the barrage site respectively, and with the last two sites being close to the  
2 barrage location. Comparisons of water levels for the whole simulation period, including for spring and  
3 neap tides, are shown in Fig. 5. It can be seen that the numerical model results agreed well with  
4 measured data, especially at Newport, although there is a 30cm -40cm under prediction at the wave  
5 crest compared with field data at the Mumbles and the Hinkley Point.

#### 6 7 4.2 Validation of tidal currents

8 Comparisons of velocities have also been made to validate the computational accuracy of the EFDC  
9 model, with field data being available for validation at sites along the South Wales coast and at  
10 Minehead [3-5]. The two sites are referred to as S1 and S2 in Fig.1. Fig. 6 illustrates the comparisons  
11 between the numerical model predictions and the observed data, which again indicate that the predicted  
12 velocities generally show good agreement with the field data for both the current speed and direction, at  
13 both sites, although there are some visible differences at the 91<sup>st</sup> and 140<sup>th</sup> hour, with 0.3 m/s and 0.2  
14 m/s respectively at site S1. Overall, the predicted results can be considered to have reproduced the field  
15 data satisfactorily. The differences between the predicted and field data were calculated and the root  
16 mean squared values for the tidal levels and currents were found to be 0.2122 and 0.1857, respectively.

17 Tidal stream data given in Admiralty Chart No. 1179 were also used to validate the numerical  
18 model predictions for the tidal currents, with comparisons between the numerical results and observed  
19 data being shown in Fig. 7 for four sites B, M, Q and U respectively. Site B is located in the outer  
20 region of the Bristol Channel and close to the seaward open boundary; sites M and Q are in the middle  
21 of the Bristol Channel and site U is very close to the proposed location of the Severn Barrage. The  
22 predicted results were in good agreement with the observed data at the four sites, both for the spring  
23 and neap tides respectively. In particular, it should be noted that the predicted current speeds and  
24 directions at Site U, close to the proposed barrage site, were very close to the observed data for both the  
25 spring and neap tides.

26 From the comparisons between the field data and the numerical model results for the water  
27 levels and tidal currents, it can be seen that EFDC model provides a reliable numerical prediction of the  
28 hydrodynamics in the Bristol Channel and Severn Estuary for establishing the impact of any proposed  
29 renewable energy project.



## 1 **5. MODEL APPLICATIONS**

2 After model validation for the natural conditions in the estuary, the EFDC model was then used to  
3 predict the hydrodynamic processes with the barrage module included, with the aim being to predict the  
4 hydrodynamic processes within the basin for a range of scenarios, both with and without the Severn  
5 Barrage included. Some of the details analysed in this section include: evaluating the changes in the  
6 maximum water levels, the minimum water depths, the maximum tidal currents and the impacts of the  
7 barrage on the salinity concentration distributions.

### 8 9 5.1 Prediction of maximum water levels

10 Due to the proposed construction and operation of the Severn Barrage across the Bristol Channel and  
11 Severn Estuary, as shown in Fig. 2, the hydrodynamic, sediment transport and water quality  
12 characteristics in the basin could be significantly affected by such a structure. The corresponding  
13 predicted changes in the maximum water levels for a mean spring tide, both with and without the  
14 barrage, are shown in Fig. 8. From these comparisons, it can be seen that there will be a very limited  
15 impact on the maximum water levels in the outer region of the Bristol Channel. However, there will be  
16 a decrease of about 0.5m in the water levels downstream of the barrage; and a significant decrease of  
17 between 0.5 and 1 m in the water levels upstream of the barrage due to the reduction in the fluid flux  
18 passing upstream of the structure with the barrage in place. These predicted results indicate that the  
19 construction of a barrage would reduce the flood risk in the region upstream of the barrage. For  
20 accurate predictions of the flood risk impact on the outer region of the Bristol Channel, then far field  
21 boundary conditions out to the Continental Shelf would need to be used.

### 22 23 5.2 Prediction of minimum water depths

24 Because of the relatively high bed elevation upstream of the barrage, the minimum water depth will be  
25 significantly affected in these regions by the construction of the Severn Barrage, which further affects  
26 the variations in the areas of intertidal zones. Fig. 9 shows a comparison of minimum water depths for  
27 the cases without and with the barrage, where it can be seen that there is no noticeable influences in the  
28 outer region of the Bristol Channel and downstream of the barrage. However, the effect of the barrage  
29 significantly affects the low water levels upstream of the structure. For the current model domain,  
30 according to the results shown in Fig. 9, the maximum areas of intertidal zones would be 257.9 km<sup>2</sup> and

1 177.4 km<sup>2</sup> for the cases without and with the Severn Barrage, respectively. So the loss of the intertidal  
2 mudflats would be 80.5 km<sup>2</sup> or 8,050 hectares, which would lead to an adverse impact on the  
3 ecological system within the estuary. Thus, some management measures need to be implemented to  
4 composite for this intertidal habitat loss.

### 5 6 5.3 Prediction of maximum current speeds

7 For the maximum tidal currents it can be seen that they are significantly reduced in the computational  
8 domain as a result of the barrage, with a comparison of the maximum tidal currents without and with  
9 the barrage being shown in Fig. 10. It can be seen that the maximum velocity is close to 2.4 m/s in the  
10 middle of the Bristol Channel without the barrage and only 1.2 m/s at the same location with the  
11 barrage. Hence the maximum velocities in the middle of the Bristol Channel and most regions in the  
12 Severn Estuary have been significantly reduced, and by as much as up to 50% in magnitude in some  
13 areas, due to the presence of a barrage. However, it can be also seen from the Fig. 10b that the  
14 maximum velocities around the sluice gates and turbines would be much greater than those for the case  
15 without the barrage, due to the barrage operation.

16

### 17 5.4 Prediction of distributions of salinity concentration

18 Due to a lack of field data for salinity concentration distributions as the boundary input condition and  
19 for model validation, the predictions of distributions of salinity are only considered as preliminary  
20 results. However, from the numerical model results it is clear that a barrage will appreciably affect the  
21 salinity distribution based on these simulations. In undertaking these simulations, the salinity level was  
22 set to 35 ppt along the open seaward boundary when water was flowing from sea into the Bristol  
23 Channel and was set as zero at the fresh water inflow source, at the tidal limit, at Gloucester. The initial  
24 conditions of salinity concentration were varied gradually from the open sea boundary to Gloucester. In  
25 order to obtain steady tidally averaged results, the simulation duration lasted for 250 days, starting at  
26 5:30pm on 20<sup>th</sup> July, 2001, and lasting up to 5:30pm on 26<sup>th</sup> March, 2002. The total CPU time for 250  
27 days of simulation was 52.37 hours. Fig. 11 shows the time series of salinity concentrations at a site  
28 close to the barrage, which indicates that the model begins to reach tidal steady state after a spin-up  
29 period of approximately 220 days. Therefore, the results after 220 days have been used to analyze the  
30 distribution of the salinity concentration distributions.

31

1 For the case without a barrage the isohalines at high water at Swansea are shown in Fig. 12 (a), from  
2 which it can be seen that the contour lines are parallel with each other, and decreasing gradually from  
3 the Bristol Channel to the Severn Estuary. The computed salinity level patterns are similar to those  
4 observed patterns for February 1978, as reported by Stephens [22] and Uncles [23]. For convenience,  
5 the observed pattern by Stephens is also shown (Fig. 12b). Similarly, the predicted isohalines at low  
6 water at Swansea are also compared with the corresponding field data, see Figs. 13 (a) and (b). At low  
7 water, some areas along the coastal lines will become dry, with these areas being marked in blue. The  
8 observed contour patterns are generally in a west\_northwest to east\_southeast direction in the most  
9 regions in the Bristol Channel at high water and lower water, however, the patterns are changed into a  
10 north\_northwest to south\_southeast direction across the water from Swansea to Minehead at lower  
11 water. It can be seen from Figs. 12 (a) and 13 (a) that the changes of the contour patterns have been  
12 largely predicted correctly using the current numerical model. The computed salinity patterns have  
13 reproduced the field data quite well both at high and low water in the Bristol Channel, as well as in the  
14 mouth of the Severn Estuary, except that the computed concentration values are a little higher than the  
15 observed values at some sites. The main reason for these differences is thought to be due to the fact that  
16 the fresh water inputs were not fully included in this model, with only the River Severn being included.  
17 At high water, the predicted salinity concentration distributions are significantly affected by the tide-  
18 dominated flow, with the corresponding salinity concentrations being higher than those at low water by  
19 1.5 - 2 ppt from the middle region of the Bristol Channel to the upstream region of the Severn Estuary,  
20 which is largely consistent with the observed salinity concentration differences between the high and  
21 low water (shown in Fig. 12 (b) and Fig. 13 (b)).

22 Due to the obstruction of the barrage, less water would enter the basin. Therefore, the salinity flux will  
23 be affected to some extent and the water upstream of the barrage will become less saline. The  
24 computed salinity patterns at high and low water at Swansea are shown for the case with a barrage in  
25 Figs. 13 (a) and (b), respectively. At high water, the salinity concentration distribution is largely  
26 unaffected by the barrage in the region between the middle of the Bristol Channel and the seaward  
27 boundary. However, the salinity concentration is reduced by 1 – 2 ppt in the regions downstream and  
28 upstream of the barrage. As the River Severn becomes increasingly narrow and meandering upstream  
29 of the Severn Bridge and the tide in the Severn Estuary is significantly weakened due to the  
30 construction of the barrage, this region will be dominated by the fresh water inputs. The predicted

1 results indicate that the salinity concentration value would be reduced by up to 5 ppt between Beachley  
2 and Sharpness. At low water, more water with lower salinity concentrations will flow out from  
3 upstream to downstream, which will dilute the salinity concentration downstream. The numerical  
4 model results show that the salinity concentrations would reduce by 0.5 – 1 ppt in the middle of the  
5 Bristol Channel and downstream and upstream of the barrage. Also, from Figs. 14 (a) and (b) it can be  
6 seen that for both high and low water the salinity concentrations upstream of the barrage are about 29  
7 ppt, and decrease gradually upwards along the Severn Estuary, and that the similar salinity  
8 concentrations of around 29 ppt can be seen downstream of the barrage. The results indicate that  
9 salinity concentrations downstream and upstream of the barrage are under a relatively stable state with  
10 a slight variation and that constructing the barrage can reduce the salt intrusion upstream of the barrage.

11 The results shown above were predicted using the 2D EFDC\_B model. In order to investigate the 3D  
12 effects of the barrage, a 3D model simulation was performed. The model had 5 vertical layers and with  
13 its horizontal grid being the same as for the 2D model. The salinity concentration obtained from the 2D  
14 model was used as the initial condition for the 3D model. At the filling stage all of the sluice gates and  
15 turbines were fully open to allow water to fill the basin, thus the 3D effects of the barrage were  
16 relatively small. However, at the generating stage water was allowed only to pass through the turbines,  
17 thus the vertical velocity was significantly affected near the turbines. Fig. 15 shows the depth averaged  
18 salinity concentration distributions for the 3D case for low water at Swansea for February, 2002. From  
19 Fig. 14(b) and Fig. 15, it can be seen that the 2D and 3D models predicted similar salinity distributions  
20 for most of the regions, including the Severn Estuary and the region between the middle of the Bristol  
21 Channel and the seaward boundary. Near the barrage site, both for the upstream and downstream sides,  
22 the 2D and 3D models predicted slightly different salinity levels, with the differences being generally  
23 less than 1 ppt. To study the 3D effects of the barrage operation in detail, the salinity concentration  
24 distributions in the surface layer and bottom layer around the barrage are shown in Fig 16. At the upper  
25 corner on the downstream side of the barrage the salinity level in the bottom layer is higher than in the  
26 surface layer. The reason for this phenomenon was caused by an eddy which formed gradually and with  
27 the size of the eddy and flow velocity increasing gradually from the bottom to the surface, which  
28 indicated that the water in the surface layer had a quicker water exchange than that in the bottom layer  
29 in that region. This effect resulted in a reduction in the salinity concentration in the surface layer.  
30 Because of the relatively low bed elevation in the region of the lower corner on the downstream side of

1 the barrage, many intertidal mudflats appeared at low water (see Fig. 16). Similar hydrodynamic  
2 conditions occurred in every vertical layer, thus the salinity concentration distributions in the water  
3 column were almost the same in the lower corner downstream of the barrage. Thus it can be concluded  
4 that the 2D model can produce similar predictions to the 3D model in most regions. If more details are  
5 needed to describe the hydrodynamics and salinity distributions in the region just downstream and  
6 upstream of the barrage, then a 3D model should be used.

7 It can be seen that the reduction in the water volume and the maximum velocity entering the Severn  
8 Estuary would influence the transportation of suspended sediments, and the corresponding bacterial and  
9 other water quality indicator levels in the water column. The relative impacts on the environmental  
10 effects of these water quality parameters are being investigated currently using the refined EFDC  
11 model. Meanwhile the potential far field impacts of the Severn Barrage on water levels and flood risk  
12 in the areas around the North Wales coast and Irish Sea are also being investigated using a larger  
13 domain and the outcome of these research studies will be published shortly.

14

## 15 **6. CONCLUSIONS**

16 In this study the EFDC model has been refined to include a barrage module for simulating tidal flows  
17 through a renewable energy barrage structure, including sluice gates and turbines. The model, including  
18 the additional barrage module (EFDC\_B), has been used to investigate the impact of the proposed  
19 Severn Barrage on the hydrodynamic and salinity processes in the Bristol Channel and Severn Estuary.  
20 This model was first validated by comparing the model predicted water levels and tidal currents with  
21 field observed data. The results indicated that the model predictions generally agreed well with the field  
22 data.

23 The EFDC\_B model was then applied to investigate the hydrodynamic processes in the estuary for a  
24 range of scenarios, both with and without the barrage, with the corresponding results showing that with  
25 the barrage the maximum water level would be significantly reduced in a large part of the Severn  
26 Estuary, especially upstream of the barrage. From the predicted minimum water depths, it has been  
27 shown that there would be a mean spring tide loss of 80.5 km<sup>2</sup> (or 8,050 hectares) of intertidal habitats  
28 due to construction of the barrage. Meanwhile, the peak tidal currents would be considerably reduced,  
29 and by as much as a half in some areas along the main channel.

1 The comparisons for the predicted salinity concentrations between the cases with and without the  
2 barrage indicated that at high water, the salinity concentration would be reduced by 1-2 ppt  
3 downstream and upstream of the barrage and in the region near Beachley the value would be reduced  
4 by up to 5 ppt, and that at low water, salinity concentrations would be reduced by 0.5 – 1 ppt in the  
5 middle of the Bristol Channel and by 0.5 ppt and 1 ppt downstream and upstream of the barrage,  
6 respectively. The predicted results also indicated that the salinity concentration values downstream and  
7 upstream of the barrage would be under a relatively stable condition with slight oscillations due to the  
8 effect of the barrage.

9 A comparison between salinity concentration distributions predicted by the 2D and 3D models  
10 indicated that they produced similar salinity distributions in the Severn Estuary and the region between  
11 the middle of the Bristol Channel and the seaward boundary. Near the barrage site, both on the  
12 upstream and downstream sides, the 2D and 3D models predicted slightly different salinity levels, with  
13 the difference being generally less than 1 ppt. Hence, in the region near to the barrage site it is  
14 preferable to use a 3D model for more detailed and accurate hydrodynamic and solute concentration  
15 distributions, as shown in recent studies by Jeffcoate *et al.* [24]

16

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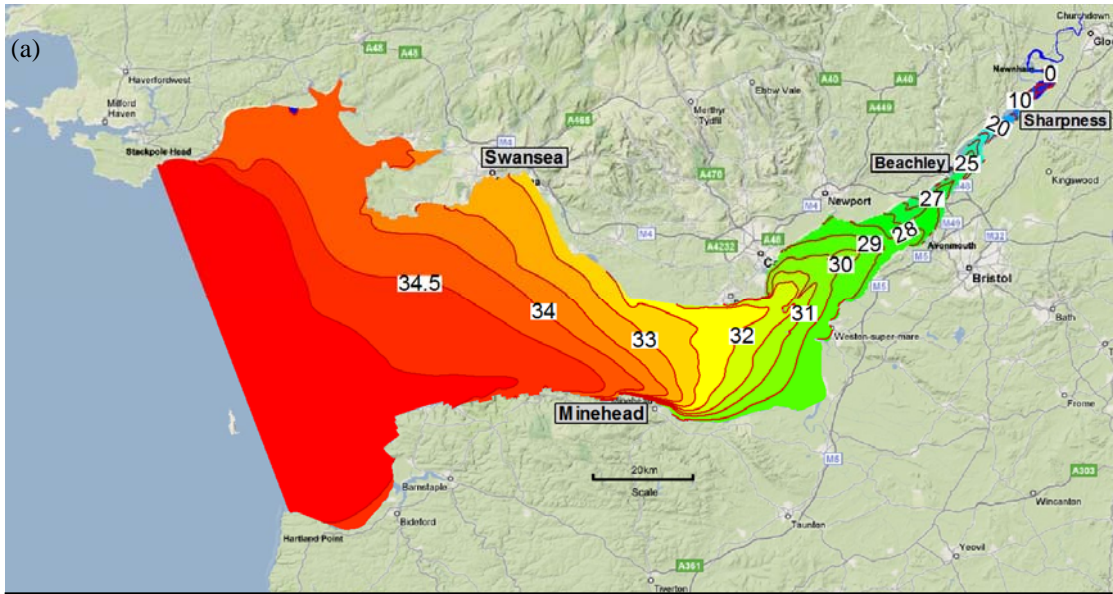
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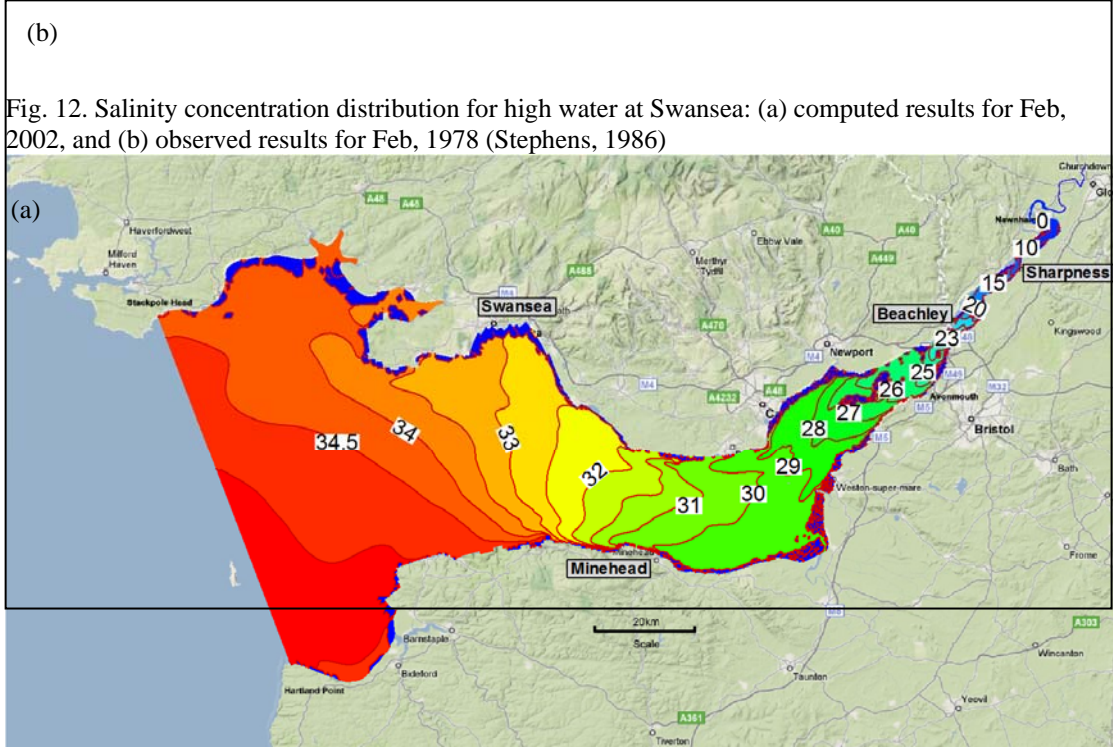
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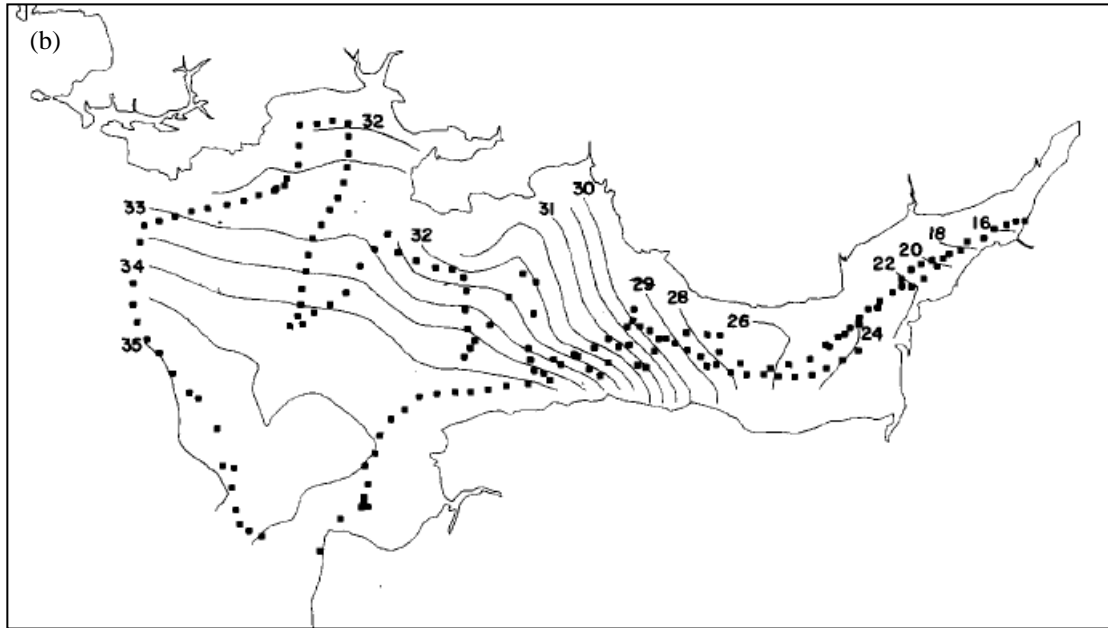




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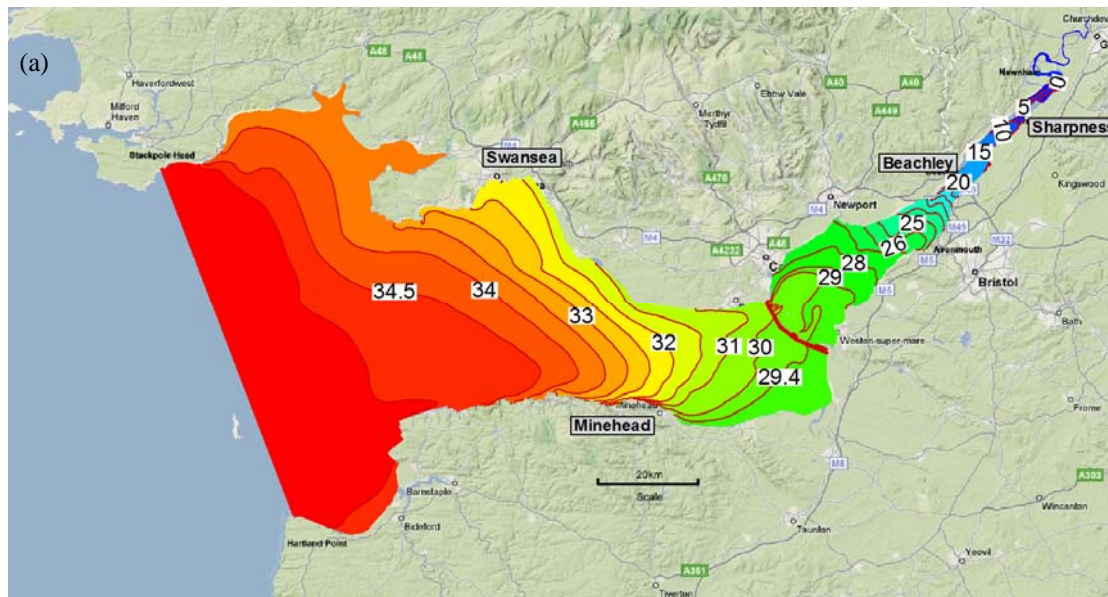


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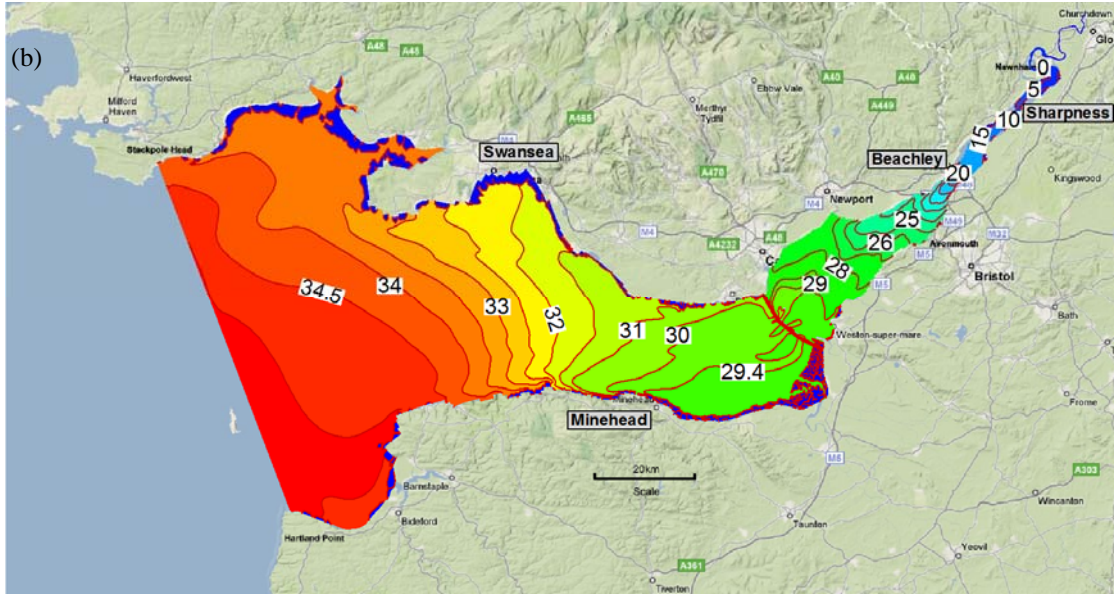


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Fig. 13. Salinity concentration distribution for low water at Swansea: (a) computed results for Feb, 2002 and (b) observed results for Feb, 1978 (Stephens, 1986)

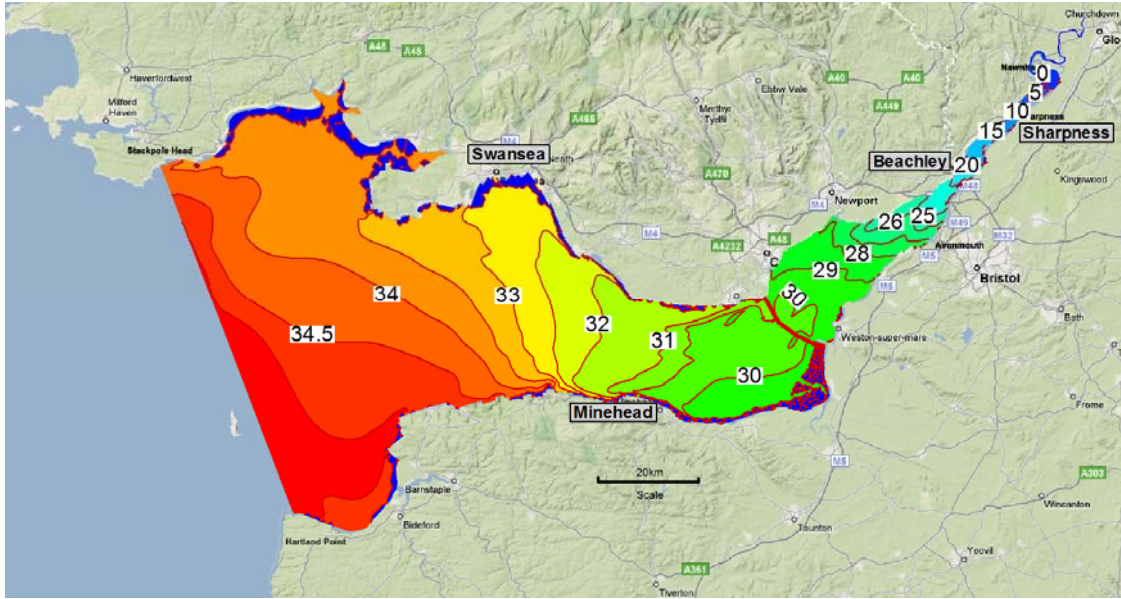


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Fig. 14. Predicted salinity concentration distribution for case with a barrage: (a) for high water at Swansea, and (b) for low water at Swansea for Feb, 2002



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Fig. 15. Model predicted 3D depth average salinity concentration distribution for the case with a barrage for low water at Swansea

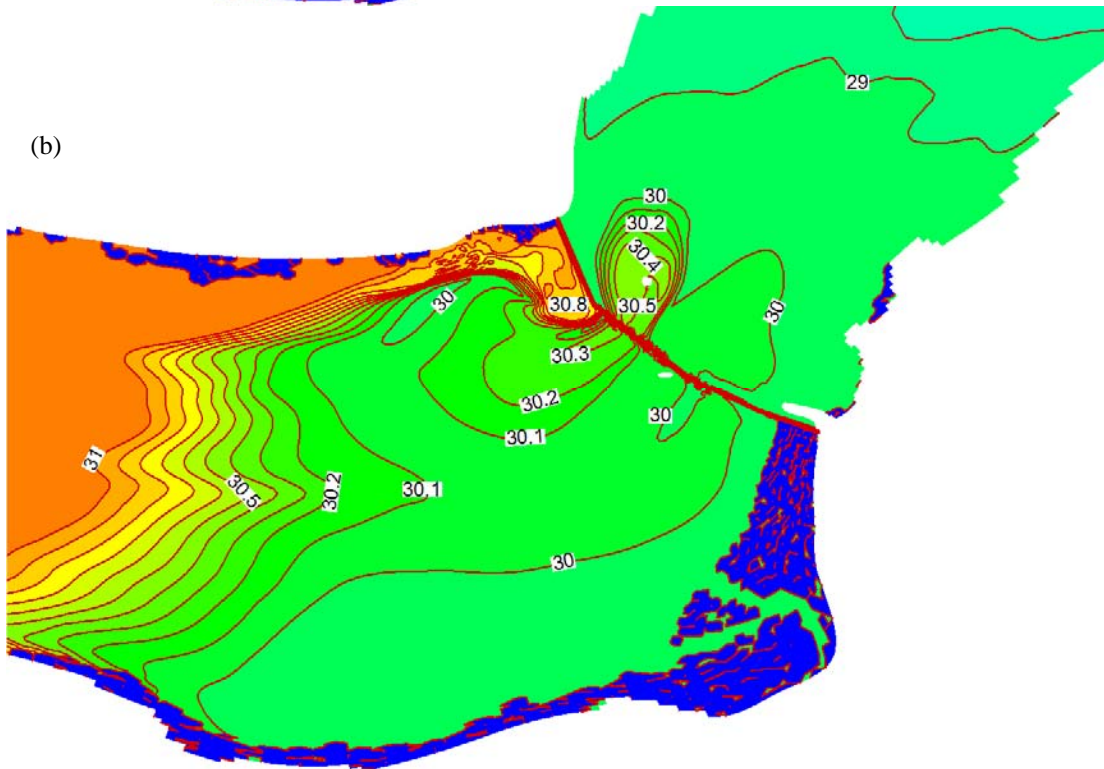
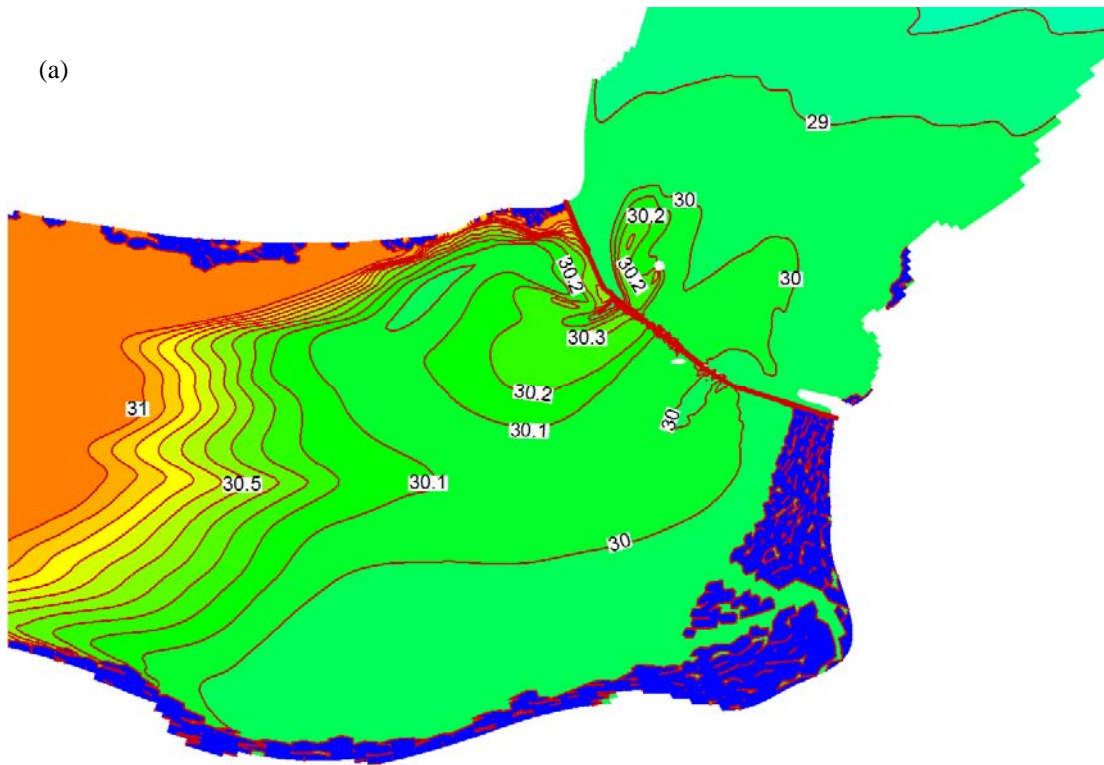


Fig. 16. A comparison between the 3D model predicted salinity concentration distributions at: (a) the surface layer, and (b) bottom layer, for the case with a barrage for low water at Swansea